

TRAFFIC IMPACT REPORT



June 11, 2024

20243889.0001

GATEWAY BUILDING DEVELOPMENT

TOWN OF HAMBURG, NY

PREPARED FOR:

3556 Lake Shore Development
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1.0 EXECUTIVE SUMMARY

The purpose of this report is to evaluate the potential traffic impacts related to the proposed Gateway Building development located at 3556 Lakeshore Road in the Town of Hamburg, NY. Within this report, the operating characteristics of the proposed access point and impacts to the adjacent roadway network are evaluated and mitigating measures are identified (if needed) to minimize operational concerns. To define traffic impact, this analysis establishes existing baseline traffic conditions, projects background traffic flow including area growth, and determines the traffic operations that would result from the proposed project.

Project Location and Description

The project site is located at 3556 Lakeshore Road in Hamburg, NY. The site is bounded by Lakeshore Road to the east, Gateway Blvd to the west and south, and single family homes along 7th St to the north. The project site is currently occupied by the existing Gateway Building. Land uses within the vicinity of the project site are generally commercial, residential, and recreational.

The proposed Gateway Building Development consists of constructing 16 five-story townhome buildings containing 8 units each with attached garages for a total of 128 townhome units. The existing seven-story building will be renovated to contain a 35-room hotel with a lobby and two restaurants on the first and second floor, ten apartments on the third, fourth, sixth, and seventh floor, and the fifth floor will contain an office, which is currently in use. Access to the site will be provided via the existing enter only, exit only, and full access driveways along Gateway Blvd, and there will be a proposed full access driveway along the southern edge of the proposed project, to the west of the exit only driveway. The overall site plan is provided at the end of this report.

Study Area

To ensure a comprehensive analysis of potential traffic impacts, a study area was selected consisting of the following two (2) intersections:

1. Gateway Blvd/Main Enter Driveway/Southtowns Office Driveway
2. Gateway Blvd/Main Exit Driveway

Existing Conditions

Turning movement traffic counts were collected by Passero Associates on Wednesday, May 29th, 2024, at the study intersection for the weekday AM and PM peak hour periods. Traffic counts were conducted between 7:00-9:00 AM for the weekday AM peak period and 4:00-6:00 PM for the weekday PM peak period. The peak hour traffic periods occurred between 7:15-8:15 AM and 4:00-5:00 PM.

Background Conditions

Background traffic volumes represent the traffic conditions during the proposed build year without development of the project. Construction of the proposed project is anticipated to reach full completion in approximately five years. The widely accepted methodology for preparing traffic impact studies requires that any projects in the study area that are currently approved and/or under construction must be considered in the traffic analysis. Projects that are contemplated but not yet approved are not included in a traffic analysis. Local municipal personnel were contacted to discuss any other specific projects that are currently approved or under construction that would generate additional traffic in the study area. No nearby projects were identified.

A review of available historical NYSDOT traffic volume data in the vicinity of the site indicates that traffic has decreased between 2010 and 2019. To account for normal increases in background traffic growth, including any unforeseen developments, a growth rate of 0.25% was applied to the existing traffic volumes for the five-year build out period.

Conclusions and Recommendations

This Traffic Impact Study identified and evaluated the potential traffic impacts that can be expected from the proposed Gateway Building development located at 3556 Lakeshore Road in the Town of Hamburg, NY. The results of this study determined that the existing transportation network can adequately accommodate the projected traffic volumes and resulting minor impacts to study area. The following sets forth the conclusions and recommendations based upon the results of the analyses:

Conclusions

1. The proposed project is expected to generate approximately 96 entering/90 exiting vehicle trips during the AM peak hour and 111 entering/140 exiting vehicle trips during the PM peak hour.
2. However, given that the office is currently in operation and that the same operations will continue once the project is complete, the proposed project is expected to generate approximately 22 entering/55 exiting new vehicle trips during the AM peak hour and 64 entering/46 exiting new vehicle trips during the PM peak hour. Based on the site generated trips for the old office land use and the projected site generated trips for the proposed project, there will be a decrease of 33 site generated trips during the AM peak hour and an increase of 35 site generated trips during the PM peak hour.
3. All movements operate at an acceptable LOS "A" under existing, projected background, and full build conditions during both peak hours studied at all study intersections.
4. No changes in level of service are anticipated during either peak hour at any of the study intersections between background and full build conditions.
5. During both the parking with special event and parking without special event parking scenarios, the anticipated parking demands can be sufficiently accommodated by the proposed parking supply.
6. The detailed analysis contained in this Traffic Impact Study demonstrates the proposed project will not result in any potentially significant adverse environmental impacts for the purpose of the environmental review of the project pursuant to the State Environmental Quality Review Act ("SEQRA").

Recommendations

7. The proposed westerly driveway along Gateway Blvd should be designed to provide one entering and one exiting lane.

2.0 INTRODUCTION

2.1 Study Purpose and Objectives

The purpose of this report is to evaluate the potential traffic impacts related to the proposed Gateway Building development located at 3556 Lakeshore Road in the Town of Hamburg, NY. Within this report, the operating characteristics of the proposed access point and impacts to the adjacent roadway network are evaluated and mitigating measures are identified (if needed) to minimize operational concerns. To define traffic impact, this analysis establishes existing baseline traffic conditions and determines the traffic operations that would result from the proposed project. All supporting calculations are included in the Appendices of this report.

2.2 Project Location

The project site is located at 3556 Lakeshore Road in Hamburg, NY. The site is bounded by Lakeshore Road to the east, Gateway Blvd to the west and south, and single family homes along 7th St to the north. The project site is currently occupied by the existing Gateway Building. Land uses within the vicinity of the project site are generally commercial, residential, and recreational.

2.3 Study Area

To ensure a comprehensive analysis of potential traffic impacts, a study area was selected consisting of the following two (2) intersections:

1. Gateway Blvd/Main Enter Driveway/Southtowns Office Driveway
2. Gateway Blvd/Main Exit Driveway

The project site location and study area are illustrated in **Figure 1** (all figures are included at the end of this report).

3.0 TRANSPORTATION SETTING

3.1 Description of Study Area Roadways

The information outlined in **Table 1** provides a description of the existing roadway network within the study area. **Figure 2** illustrates the lane geometry and traffic controls at each of the study intersections and the Annual ADT (AADT) volumes on the study roadways. The AADTs, in vehicles per day (vpd), reflect the most recently collected data obtained from the NYSDOT.

Functional classification of roadways is determined by the NYSDOT and the Federal Highway Administration (FHWA). Both the NYSDOT and FHWA group roads, streets, and highways into different classes based on how they are used. This is called functional classification. Roads and streets do not work alone to move traffic. Instead, they form a network. Functional classification defines how each road or street fits into this network, how it provides access to nearby properties, and whether it is in an urban or rural area.

In the study area, all the roadways are classified as rural. The primary functional classifications within the study area:

- Urban Local (Class 19)

Table 1: Existing Highway System

ROADWAY	CLASS ¹	AGENCY ²	SPEED LIMIT	TYPICAL CROSS SECTION ³	AADT
Gateway Blvd	19	City of Buffalo	30 mph	2-lane undivided	1,253 PASSERO (2024)

Notes:

1. Functional Classification.
2. Roadway ownership.
3. Excludes turning lanes at intersections.

3.2 Description of Multimodal Network

The following summarizes the traffic controls, pedestrian, bicycle, and transit accommodations for the study area intersections. **Figure 2** also illustrates the turn lane lengths and traffic controls at the study intersections.

Table 2: Multimodal Network

INTERSECTION	TRAFFIC CONTROL	PEDESTRIAN			BICYCLE		TRANSIT
		SIDEWALK	CROSSWALK	PED SIGNAL	LANE	OTHER	
Gateway Blvd/Main Enter Driveway/ Southtowns Office Driveway	Sign	Some presence	No	No	No	No shoulder	No stops
Gateway Blvd/Main Exit Driveway	Sign	Some presence	No	No	No	No shoulder	No stops

3.3 Planned/Programmed Highway Improvements

There are no planned/programmed highway improvement projects in the study area.

4.0 EXISTING CONDITIONS ANALYSIS

4.1 Peak Intervals for Analysis

Given the functional characteristics of the corridors, adjacent land uses, and the proposed land use for the project site, the peak hours selected for analysis are the weekday AM and weekday PM peak periods. The combination of site traffic and adjacent through traffic produces the greatest demand during these time periods.

4.2 Existing Traffic Volume Data

Turning movement traffic counts were collected by Passero Associates on Wednesday, May 29th, 2024, at the study intersection for the weekday AM and PM peak hour periods. Traffic counts were conducted between 7:00-9:00 AM for the weekday AM peak period and 4:00-6:00 PM for the weekday PM peak period. The peak hour traffic periods occurred between 7:15-8:15 AM and 4:00-5:00 PM. The existing peak hour traffic volumes are shown in **Figure 3**.

All turning movement count data was collected on a typical weekday while local schools were in session. No adverse weather conditions impacted the traffic counts. The traffic volumes were reviewed for seasonality and to confirm the accuracy.

4.3 Field Observations

The study intersections were observed during peak intervals to assess current traffic operations. This information was used to support and/or calibrate capacity analysis models described in detail later in this report.

5.0 BACKGROUND (NO BUILD) CONDITIONS

Background traffic volumes represent the traffic conditions during the proposed build year without development of the project. Construction of the proposed project is anticipated to reach full completion in approximately five years. The widely accepted methodology for preparing traffic impact studies requires that any projects in the study area that are currently approved and/or under construction must be considered in the traffic analysis. Projects that are contemplated but not yet approved are not included in a traffic analysis. Local municipal personnel were contacted to discuss any other specific projects that are currently approved or under construction that would generate additional traffic in the study area. No nearby projects were identified.

A review of available historical NYSDOT traffic volume data in the vicinity of the site indicates that traffic has decreased between 2010 and 2019. To account for normal increases in background traffic growth, including any unforeseen developments, a growth rate of 0.25% was applied to the existing traffic volumes for the five-year build out period. **Figure 4** depicts the background traffic volumes.

6.0 PROPOSED DEVELOPMENT CONDITIONS

6.1 Project Description

The proposed Gateway Building Development consists of constructing 16 five-story townhome buildings containing 8 units each with attached garages for a total of 128 townhome units. The existing seven-story building will be renovated to contain a 35-room hotel with a lobby and two restaurants on the first and second floor, ten apartments on the third, fourth, sixth, and seventh floor, and the fifth floor will contain an office, which is currently in use. Access to the site will be provided via the existing enter only, exit only, and full access driveways along Gateway Blvd, and there will be a proposed full access driveway along the southern edge of the proposed project, to the west of the exit only driveway. The overall site plan is provided at the end of this report. The overall site plan is provided at the end of this report.

6.2 Proposed Traffic Generation

The volume of traffic generated by a site is dependent on the intended land use and size of the development. Trip generation is an estimate of the number of trips generated by a specific building or land use. These trips represent the volume of traffic entering and exiting the development. *Trip Generation Manual (11th Edition)* published by the Institute of Transportation Engineers (ITE) is used as a reference for this information. The trip rate for the peak hour of the generator may or may not coincide in time or volume with the trip rate for the peak hour of adjacent street traffic. Volumes generated during the peak hour of the adjacent street traffic and proposed land use, in this case, the weekday AM and PM peak hours, represents a more critical volume when analyzing the capacity of the system; that interval will provide the basis of this analysis.

According to the ITE, the following steps are recommended when determining trip generation for proposed land uses:

- i. Check for the availability of local trip generation rates for comparable uses.

- ii. If local trip data for similar developments are not available and time and funding permit, conduct trip generation studies at sites with characteristics similar to those of the proposed development.

Traffic volume data was collected by Passero Associates at the existing Gateway Building driveways located at 3556 Lakeshore Road, Hamburg, NY. Traffic entering and exiting the driveways on the existing site was counted on Wednesday, May 29th, 2024, for the AM and PM peak hour periods. The driveway peak hour site trips documented at the existing Gateway Building driveways are an appropriate comparison of the office trips for the proposed project considering that there will be no change to the office space with the proposed project. All trip generation information has been included in the Appendices.

Table 3 shows the total site generated trips for the weekday AM and PM peak hours for the proposed development. **Table 3** also shows the total site generated trips for the development under its original land use as an office and compares those site generated trips to the proposed project's site generated trips. All trip generation information has been included in the Attachments.

Table 3: Site Generated Trips

DESCRIPTION	ITE LUC ¹	SIZE	AM PEAK HOUR		PM PEAK HOUR	
			ENTER	EXIT	ENTER	EXIT
Hotel	310	±35 Rooms	6	4	11	10
Multifamily Housing (Mid-Rise)	221	±40 Units	1	5	10	6
Single-Family Attached Housing	215	±128 Units	15	46	43	30
Office ²	Local Data	±13,000 SF	74	35	47	94
Total Site Generated Trips			96	90	111	140
Total New Site Generated Trips			22	55	64	46
Office	710	±137,000 SF	193	26	37	179
Difference In Site Generated Trips			-97	+64	+74	-39
Note:						
1. LUC = Land Use Code.						
2. Trips for the office land use are existing and are not new trips generated by the project						

The proposed project is expected to generate approximately 96 entering/90 exiting vehicle trips during the AM peak hour and 111 entering/140 exiting vehicle trips during the PM peak hour. However, given that the office is currently in operation and that the same operations will continue once the project is complete, the proposed project is expected to generate approximately 22 entering/55 exiting new vehicle trips during the AM peak hour and 64 entering/46 exiting new vehicle trips during the PM peak hour. Based on the site generated trips for the old office land use and the projected site generated trips for the proposed project, there will be a decrease of 33 site generated trips during the AM peak hour and an increase of 35 site generated trips during the PM peak hour.

6.3 Trip Distribution

The cumulative effect of site-generated traffic on the transportation network is dependent on the origins and destinations of that traffic and the location of the access drives serving the site. The proposed arrival/departure distribution of traffic generated by the proposed project is considered a function of several parameters, including:

- Residential and employment centers using U.S. Census Data
- Nearby commercial centers

- Proximity to local schools
- Site driveway locations
- Existing traffic patterns
- Existing traffic conditions and controls

Figure 5 shows the anticipated trip distribution pattern percentage for the project site. **Figure 6** shows the total site generated trips based on the distribution patterns.

6.4 Full Development Volumes

The proposed design hour traffic volumes are developed for the peak hours by combining the background traffic conditions (**Figure 4**) and the new site-generated traffic volumes (**Figure 6**) to yield the traffic volumes under full development conditions. **Figure 7** illustrates the full build traffic conditions.

7.0 TRAFFIC OPERATIONS AND ANALYSIS

7.1 Description of Capacity Analysis

Capacity analysis is a technique used for determining a measure of effectiveness for a section of roadway and/or intersection based on the number of vehicles during a specific time period. The measure of effectiveness used for the capacity analysis is referred to as a Level of Service (LOS). Levels of service are calculated to provide an indication of the amount of delay that a motorist experiences while traveling along a roadway or through an intersection. Since the most amount of delay to motorists usually occurs at intersections, capacity analysis focuses on intersections, as opposed to highway segments.

The standard procedure for capacity analysis of signalized and unsignalized intersections is outlined in the *Highway Capacity Manual (HCM) 7th Edition* published by the TRB. Traffic analysis software, Synchro 12, which is based on procedures and methodologies contained in the HCM, was used to analyze operating conditions at study area intersections. The procedure yields a level of service based on the HCM as an indicator of how well intersections operate.

Six levels of service are defined for analysis purposes. They are assigned letter designations, from "A" to "F", with LOS "A" representing the conditions with little to no delay, and LOS "F" conditions with very long delays. LOS "C" or better is desirable, but LOS "D" for signalized locations and LOS "E" for unsignalized locations are generally thresholds of acceptable operation during peak periods so long as the volume to capacity ratio (v/c) is below 1.0. **Table 4** depicts level of service criteria for both signalized and unsignalized intersections.

Table 4: Level of Service Criteria

LEVEL OF SERVICE	SIGNALIZED CONTROL DELAY PER VEHICLE (seconds)	STOP CONTROL DELAY PER VEHICLE (seconds)
A	< 10	< 10
B	10 – 20	10 – 15
C	20 – 35	15 – 25
D	35 – 55	25 – 35
E	55 – 80	35 – 50
F	> 80	> 50

LOS for signalized intersections is defined in terms of delay specifically, average total delay per vehicle for a 15-minute analysis period. LOS for unsignalized intersections, however, are different from a signalized intersection. The primary reason for this is driver expectation that a signalized intersection is designed to carry higher volumes than an unsignalized intersection. Unsignalized intersections are also associated with more uncertainty for users, as delays are less predictable than they are at signals.

The v/c ratio, also referred to as degree of saturation, represents the sufficiency of an intersection to accommodate the vehicular demand. A v/c ratio less than 0.85 generally indicates that adequate capacity is available, and vehicles are not expected to experience significant queues and delays. As the v/c ratio approaches 1.0, traffic flow may become unstable, and delay and queuing conditions may occur.

7.2 Capacity Analysis Results

Existing and background operating conditions during the peak study periods are evaluated to determine a basis for comparison with the projected future conditions. Future traffic conditions generated by the project are analyzed to assess the operation of the study area intersections. **Table 5** describes the capacity results for existing, background, and full development conditions. The discussion following the table summarizes capacity conditions.

INTERSECTION	2024 EXISTING BASE CONDITIONS		2029 BACKGROUND CONDITIONS		2029 FULL BUILD CONDITIONS	
	AM	PM	AM	PM	AM	PM
2. Gateway Blvd/Proposed Driveway (U)						
SB - Proposed Driveway	N/A	—	N/A	—	N/A	—
3. Gateway Blvd/Exiting Driveway (U)						
SB - Exiting Driveway	A	8.9	A	9.3	A	8.9
4. Gateway Blvd/Entering Driveway/Southtowns Driveway (U)						
EB Left - Gateway Blvd	A	0.0	A	7.3	A	0.0
WB Left - Gateway Blvd	A	7.6	A	7.8	A	7.6
NB - Southtowns Driveway	A	8.7	A	9.1	A	8.7
					A	9.1
					A	9.5

Notes:

1. A(2.8) = Level of Service (Delay in seconds per vehicle)
2. NB = Northbound, SB = Southbound, EB = Eastbound, WB = Westbound
3. (S) = Signalized; (U) = Unsignalized
4. N/A = Approach does not exist and/or was not analyzed during this condition
5. Green shaded cells indicate low delays, yellow shaded cells indicate moderate delays, red shaded cells indicate long delays.

Gateway Blvd/Proposed Westerly Driveway

All approaches operate at LOS "A" under full development conditions during both peak hours. No improvements are warranted nor recommended at this location. The proposed driveway should consist of one entering and one exiting lane.

Gateway Blvd/Exiting Driveway

The southbound approach operates at LOS "A" under all conditions during both peak hours. No changes in level of service are anticipated and no improvements are warranted nor recommended at this location.

Gateway Blvd/Entering Driveway/Southtowns Driveway

All approaches operate at LOS "A" under all conditions during both peak hours. No changes in level of service are anticipated and no improvements are warranted nor recommended at this location.

8.0 SHARED PARKING EVALUATION

Shared parking studies are conducted to establish the total number of spaces necessary by mixed-use developments to effectively serve expected parking demands. The shared parking concept builds upon the premise that land uses in a mixed-use development often do not share the same peak demand period, so spaces can be shared between the different land uses during different peak periods.

Each land use typically has a peak demand period where it would occupy the maximum number of spaces that the use requires and an off-peak period where a lesser percentage of the maximum spaces would be occupied; be it by time of day, day of week, or even month of the year. This allows for the project to provide fewer spaces than would be required if the land uses on a project site were to be treated separately with individual parking demands. The concept of shared parking is well recognized within the real estate and regulatory community and is proven to work.

To estimate the number of parking spaces required for the proposed project, this assessment used the ULI methodology for shared parking. This methodology is utilized by transportation engineers and planners when evaluating the parking demand for a mixed-use project. The ULI *Shared Parking (3rd Edition)* includes state-of-the-art practice methodologies for determining parking demand in these types of projects.

Accompanying the publication is an interactive Shared Parking Calculation Model (Model) that is used to estimate the shared parking demand. The Model requires a user to input the number of units associated with each proposed land use. Within the Model, 32 land uses are identified – some of which are subdivided into more refined categories – with 44 different recommended base parking ratios based on suburban locations with little or no transit. Data contained within the Model is from a combination of ULI surveys and the ITE *Parking Generation Manual*. Outputs consist of a summary table describing the base parking demand and shared parking reduction; a monthly demand comparison; weekday and weekend demand by month; and weekday and weekend demand by hour.

Two parking scenarios were developed. One scenario (Scenario 1) assessed the site on a typical day without a special event. The second scenario (Scenario 2) assessed the site should a special event occur, such as a banquet or convention.

The proposed parking supply is 966 spaces. This study accounts for the garage spaces associated with the residential units and removes them from shared parking considerations.

- 16 indoor spaces
- 256 garage spaces

- 694 open spaces

Scenario 1

The peak hour demand, respective to weekday, weekend, and seasonal demands, is projected to occur at 10:00 AM on a March weekday. The projected peak hour demand for full build-out of the site (i.e., the busiest hour of the busiest weekday or weekend of the year) is ± 339 spaces. Parking demand accumulations for the peak weekday and weekend periods are presented in **Table 6** on the following page.

Table 6: *Shared Parking Demand without Special Event*

Land Use	Size	Weekday		Weekend	
		Max Demand	Shared Demand	Max Demand	Shared Demand
Residential	168 units	296	273	305	302
Office	13,000 SF	50	49	6	0
Hotel	35 keys	40	17	40	18
Total Parking Demands		386	339	351	320

Weekday shared demand occurs in March at 10:00 AM.

Weekend shared demand occurs in March at 10:00 PM.

Scenario 2

The peak hour demand, respective to weekday, weekend, and seasonal demands, is projected to occur at 10:00 AM on a February weekday. The projected peak hour demand for full build-out of the site (i.e., the busiest hour of the busiest weekday or weekend of the year) is ± 765 spaces. Parking demand accumulations for the peak weekday and weekend periods are presented in **Table 7**.

Table 7: *Shared Parking Demand with Special Event*

Land Use	Size	Weekday		Weekend	
		Max Demand	Shared Demand	Max Demand	Shared Demand
Residential	168 units	296	273	305	277
Office	13,000 SF	50	49	6	6
Hotel	35 keys	40	16	40	16
Special Event	72,000 SF	432	427	432	427
Total Parking Demands		818	765	783	726

Weekday shared demand occurs in February at 10:00 AM.

Weekend shared demand occurs in February at 11:00 AM.

During both parking scenarios, the anticipated parking demands can be sufficiently accommodated by the proposed parking supply.

9.0 CONCLUSIONS AND RECOMMENDATIONS

This Traffic Impact Study identified and evaluated the potential traffic impacts that can be expected from the proposed Gateway Building development located at 3556 Lakeshore Road in the Town of Hamburg, NY. The results of this study determined that the existing transportation network can adequately accommodate the projected traffic volumes and resulting minor impacts to study area. The following sets forth the conclusions and recommendations based upon the results of the analyses:

Conclusions

1. The proposed project is expected to generate approximately 96 entering/90 exiting vehicle trips during the AM peak hour and 111 entering/140 exiting vehicle trips during the PM peak hour.
2. However, given that the office is currently in operation and that the same operations will continue once the project is complete, the proposed project is expected to generate approximately 22 entering/55 exiting new vehicle trips during the AM peak hour and 64 entering/46 exiting new vehicle trips during the PM peak hour. Based on the site generated trips for the old office land use and the projected site generated trips for the proposed project, there will be a decrease of 33 site generated trips during the AM peak hour and an increase of 35 site generated trips during the PM peak hour.
3. All movements operate at an acceptable LOS "A" under existing, projected background, and full build conditions during both peak hours studied at all study intersections.
4. No changes in level of service are anticipated during either peak hour at any of the study intersections between background and full build conditions.
5. During both the parking with special event and parking without special event parking scenarios, the anticipated parking demands can be sufficiently accommodated by the proposed parking supply.
6. The detailed analysis contained in this Traffic Impact Study demonstrates the proposed project will not result in any potentially significant adverse environmental impacts for the purpose of the environmental review of the project pursuant to the State Environmental Quality Review Act ("SEQRA").

Recommendations

7. The proposed westerly driveway along Gateway Blvd should be designed to provide one entering and one exiting lane.

10.0 REFERENCES

- Synchro 12 Software. Cubic ITS. 2023.
- Highway Capacity Manual (7th Edition). Transportation Research Board (TRB). Washington, DC. 2022.
- Trip Generation Manual (11th Edition). Institute of Transportation Engineers (ITE). Washington, DC. 2021.
- OnTheMap. US Census Bureau. 2023.
- Traffic Data Viewer. New York State Department of Transportation (NYSDOT). 2023.
- Manual on Uniform Traffic Control Devices (MUTCD). Federal Highway Administration. 2009.
- Highway Functional Classification Concepts, Criteria, and Procedures. FHWA. 2013.
- Parking Generation Manual (6th Edition). Institute of Transportation Engineers (ITE). 2023.
- Shared Parking Model (3rd Edition). Urban Land Institute (ULI). 2020.

11.0 FIGURES

Figures 1 through 7 are included on the following pages.

CARMINWOOD DESIGN

Buffalo | Utica | Greensboro

Proposed Townhomes

3556 Lake Shore Road
Hamburg, New York

PRELIMINARY
NOT FOR CONSTRUCTION

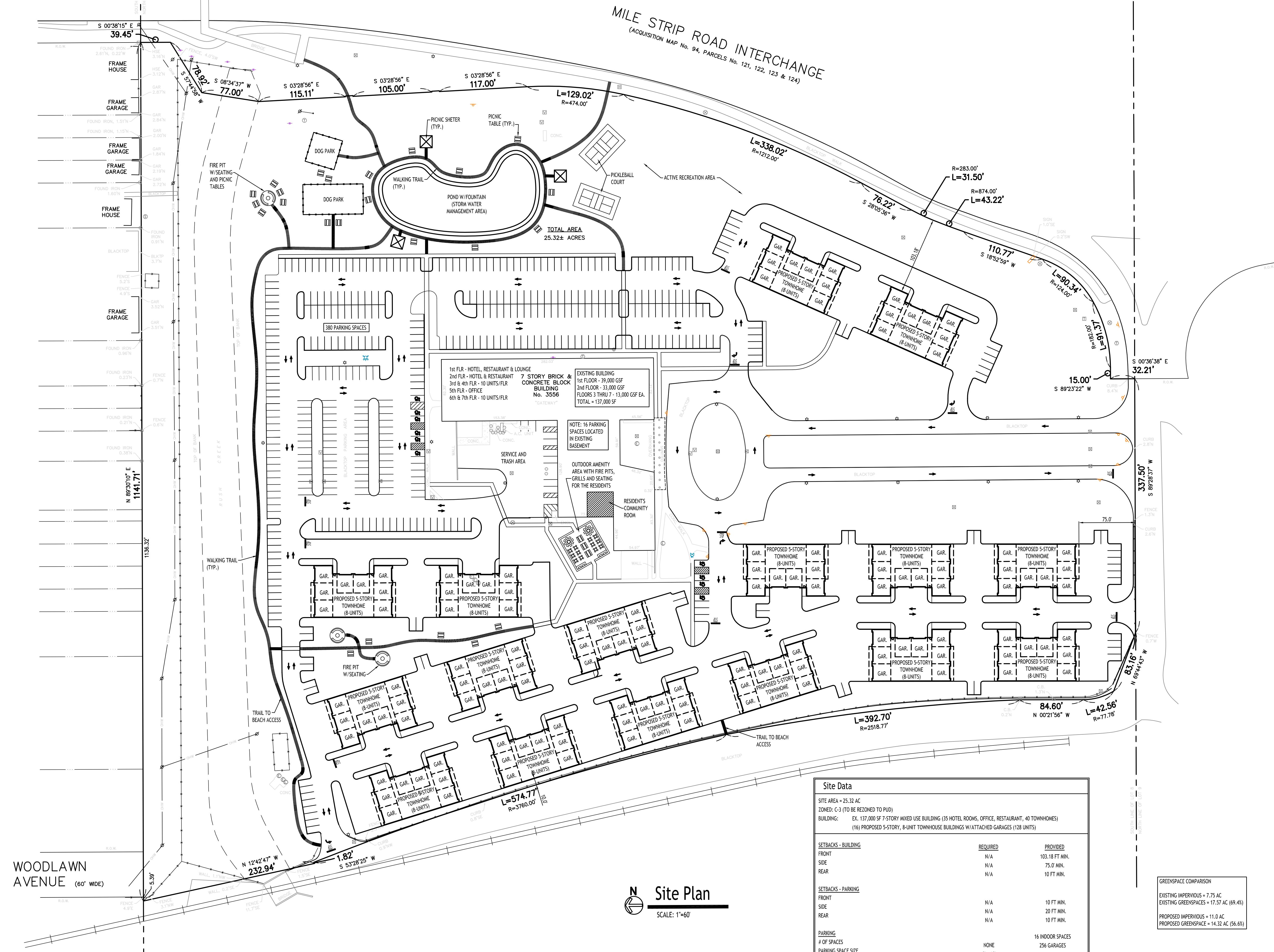


Figure 1



Gateway Building Development | Town of Hamburg, Erie County, NY

0 300 600 1,200 Feet 

Site Location and Study Area

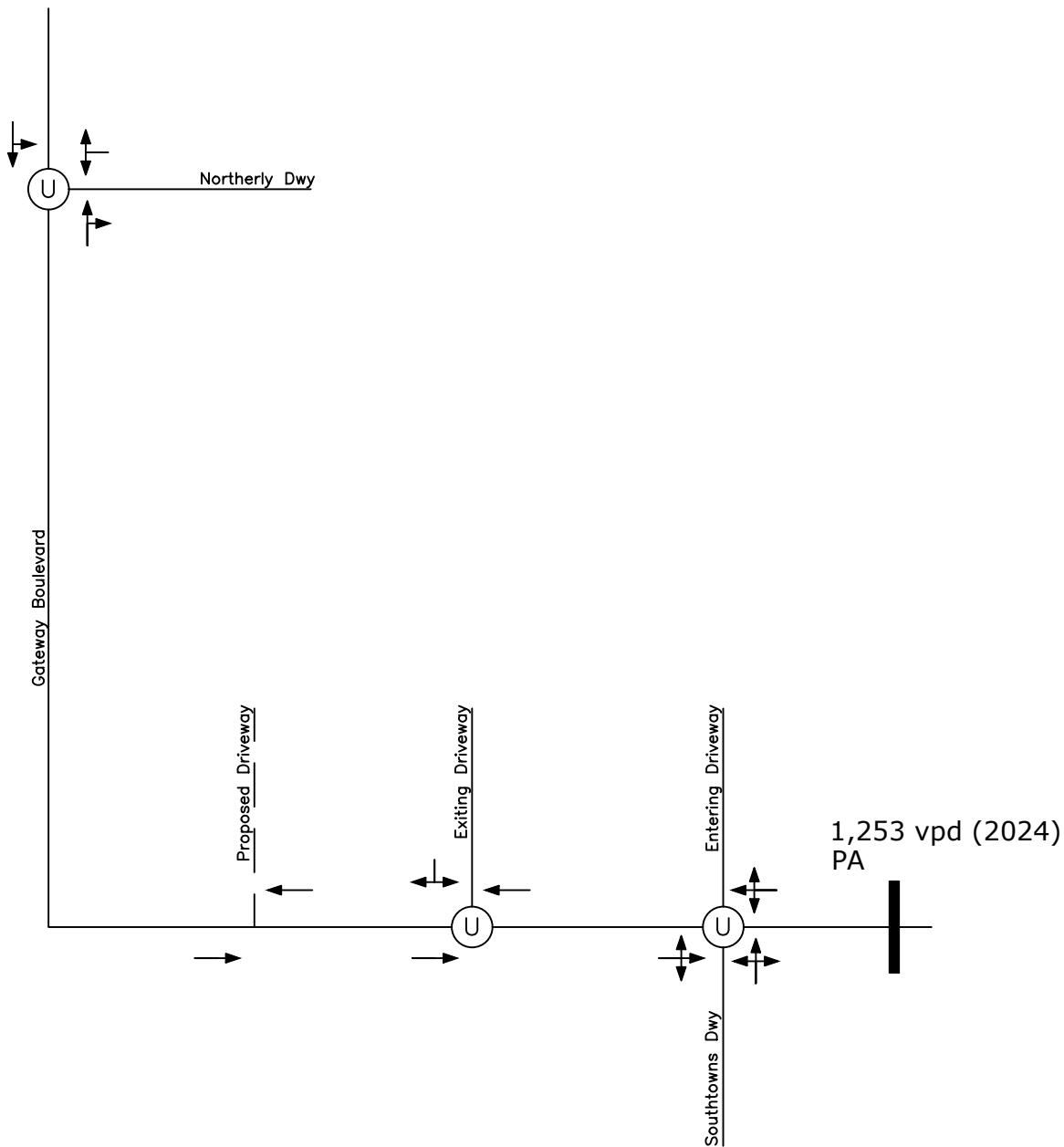
Key:

- # Study Intersection
- # Study/Proposed Intersection
- Study Area

Figure 2

Notes:

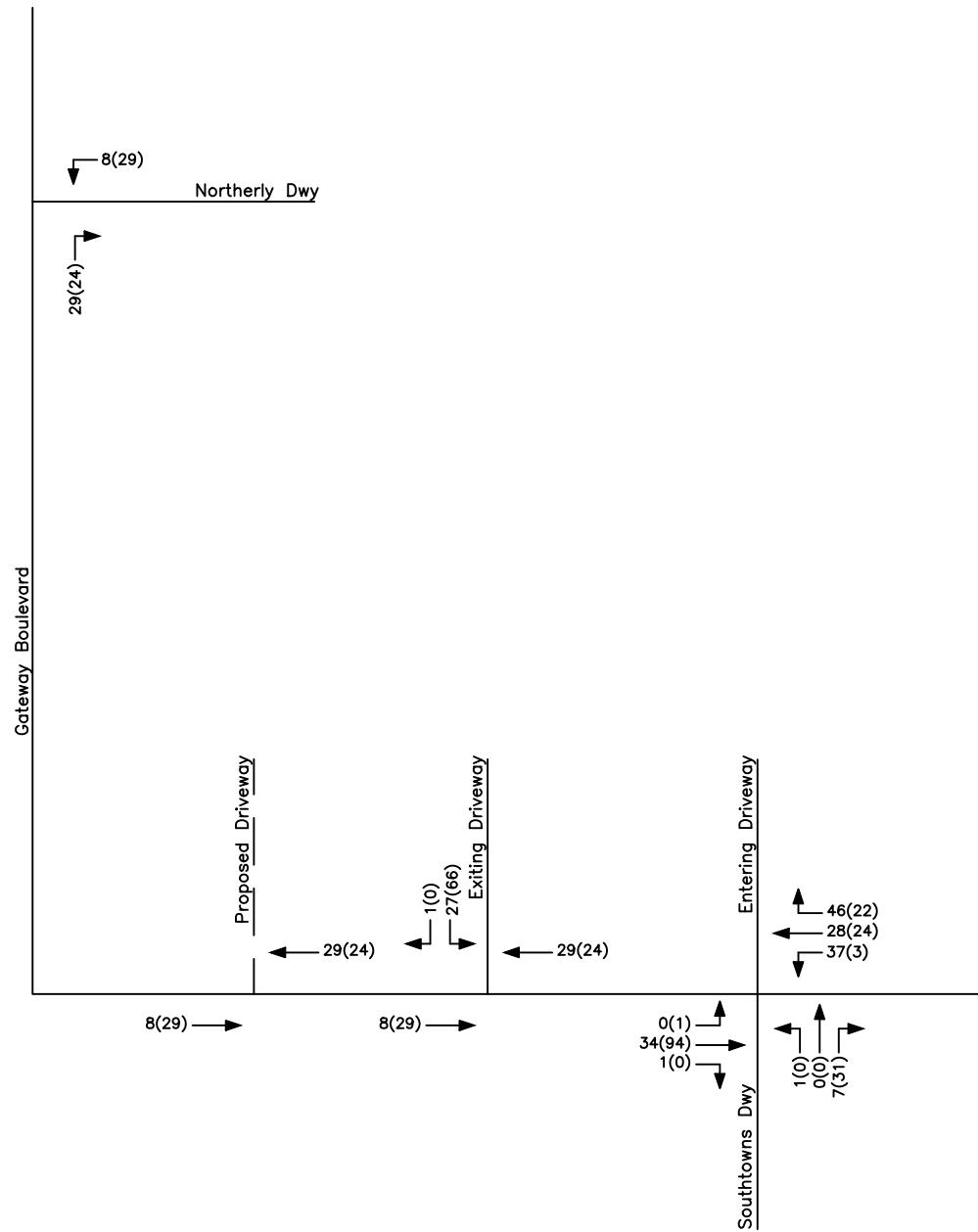
1. All AADT volumes by those noted:
 - 1.1. NYSDOT = New York State Department of Transportation.
 - 1.2. PA = Passero Associates.
2. vpd = Vehicles per day.
3. Turn lane lengths shown include only storage.



Gateway Building Development, Town of Hamburg, NY

**Lane Geometry and
Average Daily Traffic**

Figure 3

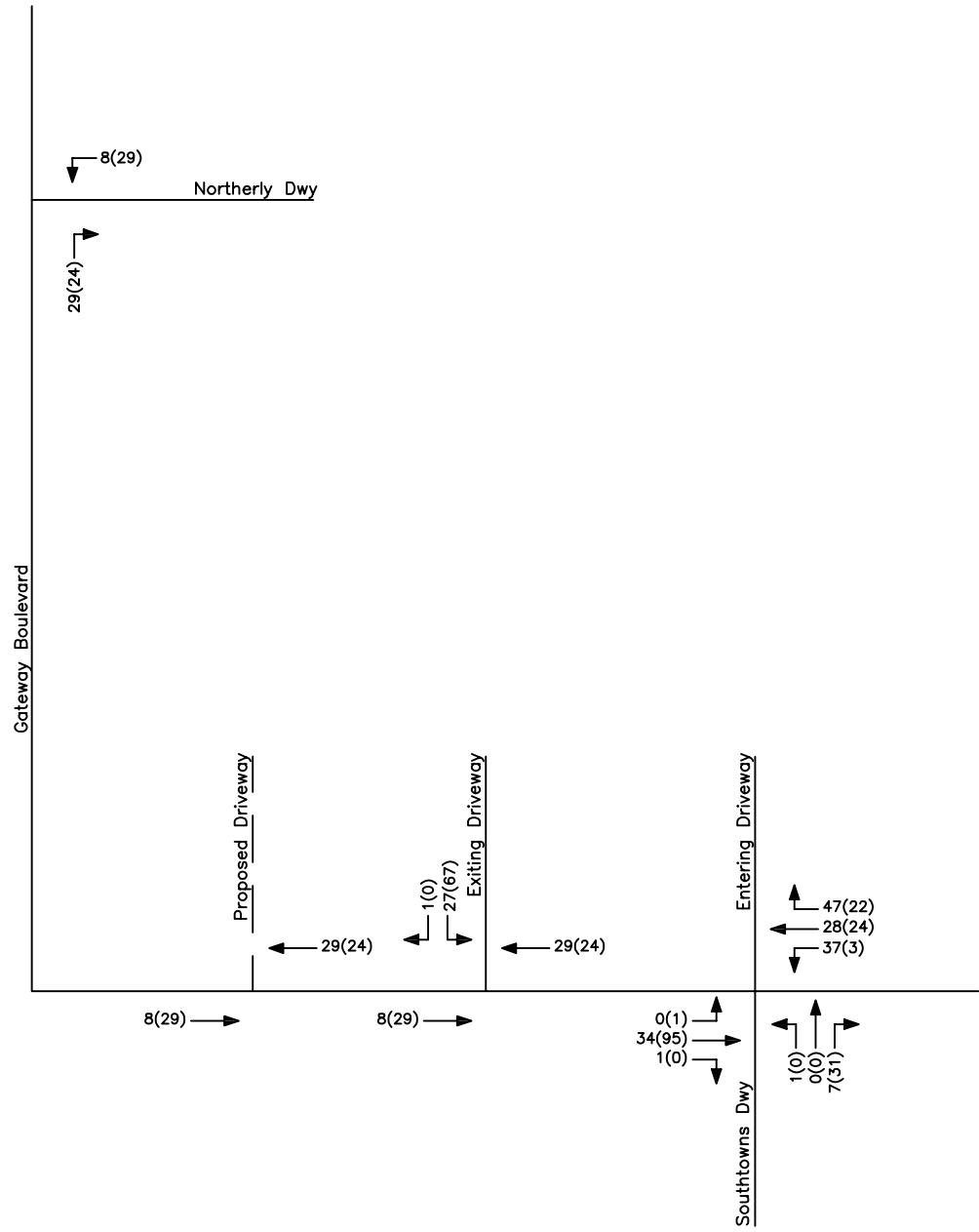


Gateway Building Development, Town of Hamburg, NY

Peak Hour Volumes
Existing Conditions

KEY:
00(00) = AM(PM)

Figure 4



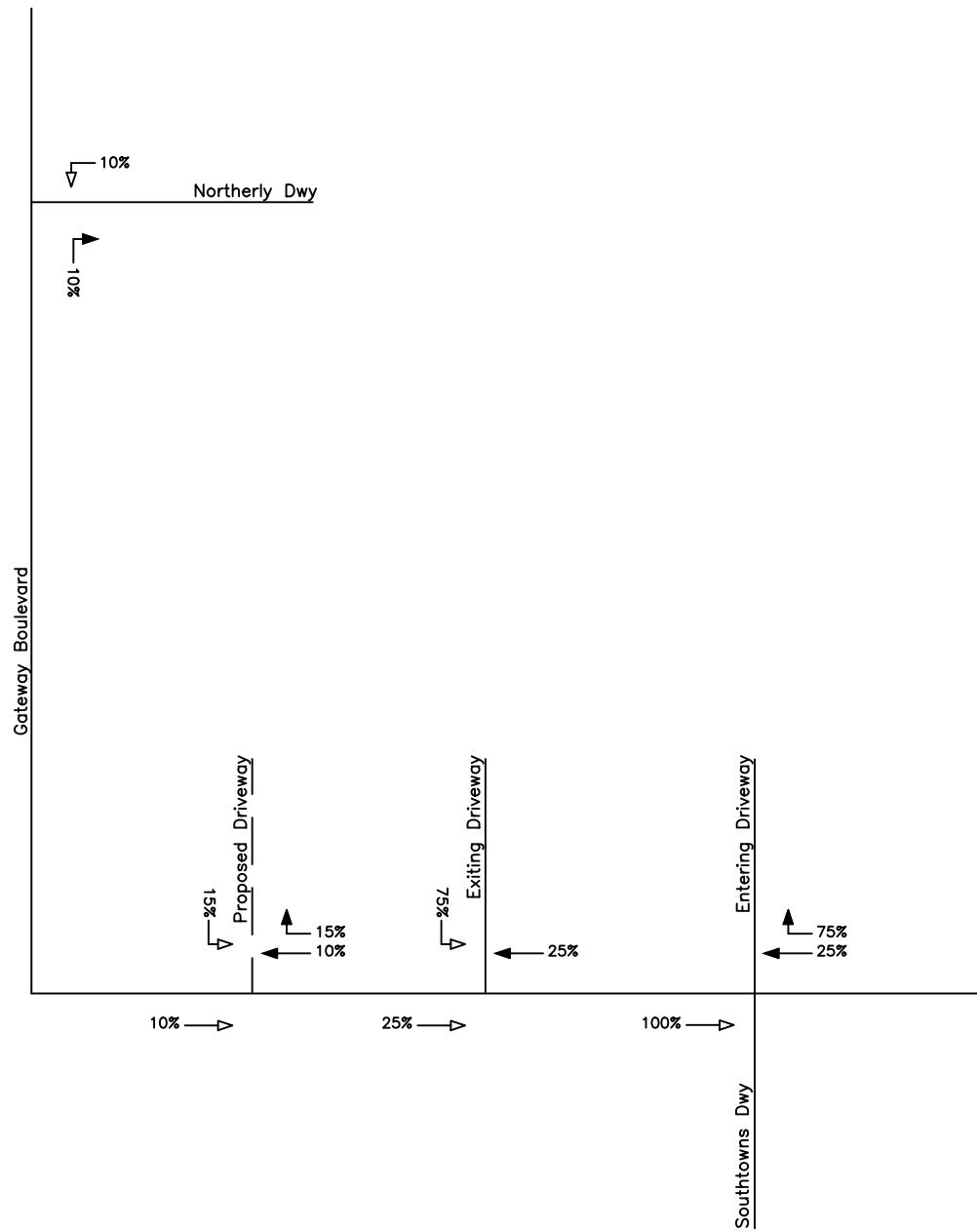
Gateway Building Development, Town of Hamburg, NY

Peak Hour Volumes
2029 Background Conditions

KEY:
00(00) = AM(PM)

↑
N
NOT TO SCALE

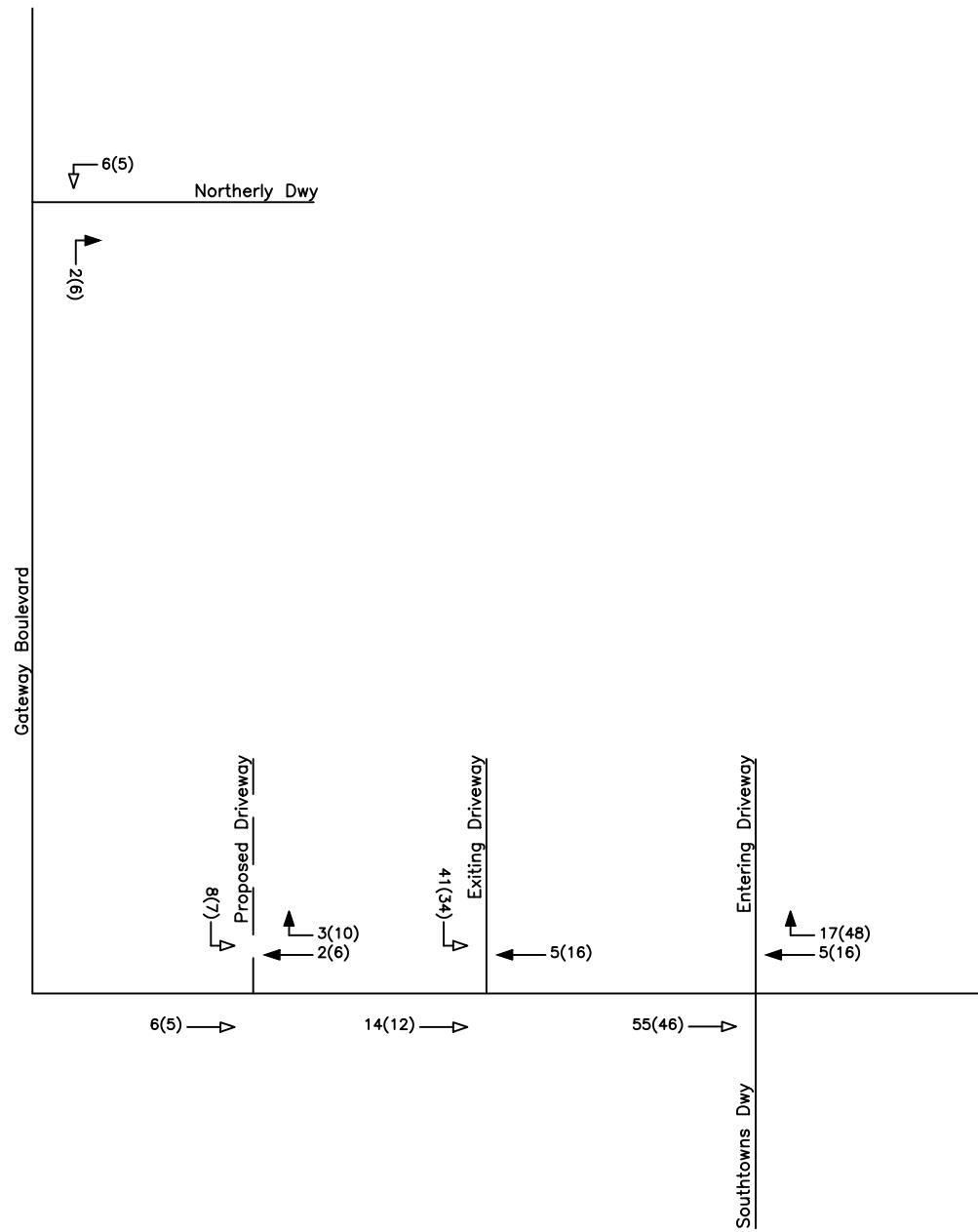
Figure 5



Gateway Building Development, Town of Hamburg, NY

Trip Distribution

Figure 6

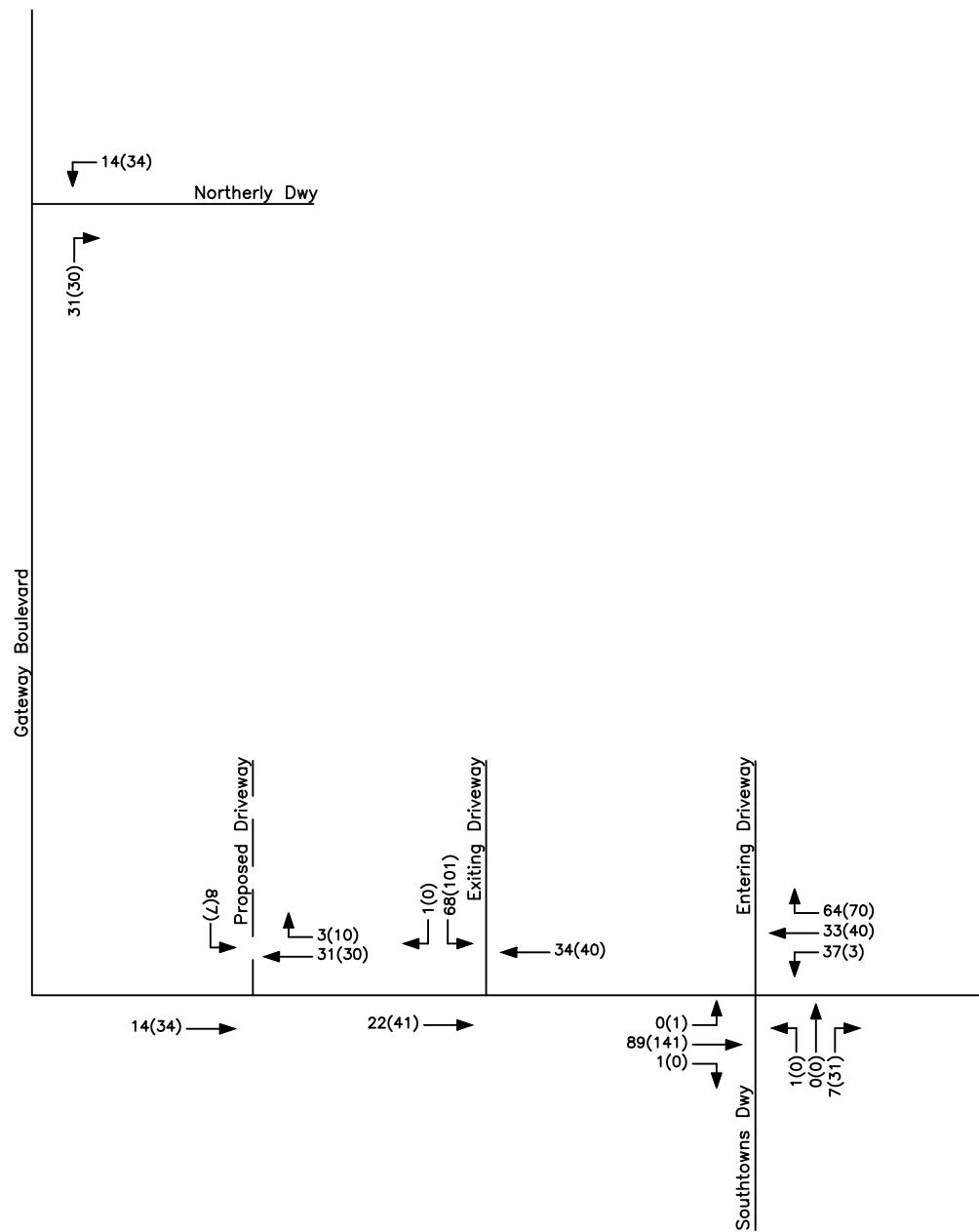


Gateway Building Development, Town of Hamburg, NY

Site Generated Trips

↑
N
NOT TO SCALE

Figure 7



Gateway Building Development, Town of Hamburg, NY

Peak Hour Volumes
Full Build Conditions

KEY:
00(00) = AM(PM)

↑
N
NOT TO SCALE

APPENDICES

APPENDIX A: EXISTING TRAFFIC COUNT DATA

Passero Associates

Reviewer: DBD

<u>Location:</u>		Project: Gateway Building																
<u>Gateway Building Driveways</u>		SB (from North)				WB (from East)				NB (from South)				EB (from West)				
VEHICLES		EC Office driveway (south leg)				Gateway Blvd (west leg)				Gateway Driveways (north leg)				Gateway Blvd (east leg)				
RT	Thru	LT	RT	Thru	LT	RT	Thru	LT	RT	Thru	LT	RT	Thru	LT	RT	Thru	LT	
0700-0715	0	5	9	1	4	0	0	0	0	0	0	0	0	0	0	0	0	19
0715-0730	0	1	6	18	10	8	0	0	0	0	0	0	3	0	0	0	0	46
0730-0745	0	10	14	9	13	2	0	0	0	0	0	0	2	0	0	0	0	50
0745-0800	1	6	9	6	11	3	0	0	0	0	0	0	1	0	0	0	0	37
0800-0815	0	4	5	3	5	2	0	0	1	0	0	0	2	0	0	0	0	22
0815-0830	0	0	3	4	4	3	0	0	0	0	0	0	1	0	0	0	0	124
0830-0845	0	3	5	1	1	0	0	0	0	0	0	0	0	0	0	0	0	15
0845-0900	0	5	8	1	2	0	0	0	0	0	0	0	0	0	0	0	0	84
Peak Hour (7:15-8:15)	1	1	26	46	28	37	7	0	1	0	0	0	0	0	0	0	0	21
Bicycles in Yellow	0%	8%	4%	0%	24%	0%	#DIV/0!	0%	#DIV/0!	0%	#DIV/0!	0%	#DIV/0!	0%	#DIV/0!	0%	0.78	

Location:

<u>Gateway Building Driveways</u>		Project: #2024-3889.0001															
		SB (from North)				WB (from East)				NB (from South)				EB (from West)			
		EC Office driveway (south leg)				Gateway Blvd (west leg)				Gateway Driveways (north leg)				Gateway Blvd (east leg)			
RT	Thru	LT	RT	Thru	LT	RT	Thru	LT	RT	Thru	LT	RT	Thru	LT	RT	Thru	LT
1600-1615	0	21	6	5	1	20	0	0	0	0	0	0	7	0	60	0	41
1615-1630	0	15	10	8	0	0	0	0	0	0	0	0	7	1	0	0	52
1630-1645	0	22	4	7	2	9	0	0	0	0	0	0	8	0	0	0	22
1645-1700	0	8	2	4	0	2	0	0	0	0	0	0	6	0	0	0	175
1700-1715	0	6	1	2	1	3	0	0	0	0	0	0	3	0	0	0	131
1715-1730	0	3	1	1	0	1	0	0	0	0	0	0	0	0	0	0	6
1730-1745	0	0	1	1	0	2	0	0	0	0	0	0	2	0	0	0	50
1745-1800	0	3	1	3	0	0	0	0	0	0	0	0	3	0	0	0	6
Peak Hour (16:00-17:00)	0	0	66	22	24	3	31	0	0	0	0	0	28	1	0	0	38
Bicycles in Yellow	#DIV/0!	#DIV/0!	2%	0%	4%	33%	3%	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	7%	0%	0.73		

Passero Associates		5/29/2024 weekday PM		SB (from North) EC Office driveway (south leg)		WB (from East) Gateway Blvd (west leg)		NB (from South) Gateway Driveways (north leg)		EB (from West) Gateway Blvd (east leg)		Reviewer: DBD
Location:	Project:			RT	Thru	RT	Thru	RT	Thru	RT	Thru	LT
Gateway Building	Gateway Building	0700-0715						1				
Gateway Building	Gateway Building	0715-0730				1	1			2		
Gateway Building	Gateway Building	0730-0745										
Gateway Building	Gateway Building	0745-0800				1	1			4		
HEAVY TRUCKS ONLY												
5/29/2024	Weekday AM	0800-0815								3		
5/29/2024	Weekday AM	0815-0830								2		
5/29/2024	Weekday AM	0830-0845										
5/29/2024	Weekday AM	0845-0900				2	2		1	1		
Peak Hour (7:15-8:15)												
5/29/2024	Weekday AM	0	0	2	2	0	0	9	0	0	0	0

Location:	5/29/2024		WB (from North)			WB (from East)			NB (from South)			EB (from West)		
	weekday	PM	EC Office driveway (south leg)	Gateway Blvd (west leg)	Gateway Driveways (north leg)	RT	Thru	LT	RT	Thru	LT	RT	Thru	LT
Gateway Building Driveways			1600-1615											
			1615-1630											
			1630-1645											
			1645-1700											
			1700-1715											
			1715-1730											
			1730-1745											
			1745-1800											
Util vehicle EB RT via grass HEAVY TRUCKS ONLY	Peak Hour (16:00-17:00)		0	0	1	0	1	1	1	0	0	1	2	0

APPENDIX B: MISCELLANEOUS CALCULATIONS

Gateway Building Development, Town of Hamburg, NY
Documentation of Ambient Traffic Volume Growth

Roadway	Segment starts at	Segment end at	2010	2011	2012	2013	2014	2015	2016	2017	2018	2019	Annual Growth
NY-5	Rte 179	City of Lackawanna	39,916	40,566					43,049			37,681	-0.64%
												AVERAGE	-0.64%

PROJECT: Gateway Building Development

LOCATION: Town of Hamburg, NY

PEAK HOUR: Weekday AM

Figure Number

PROJECT: Gateway Building Development

LOCATION: Town of Hamburg, NY

PEAK HOUR: Weekday PM

Figure Number

Figure Number		3A	4	5	6	7			
LOCATION NUMBER	INTERSECTION DESCRIPTION	2024 Collected Volumes	No Build Volumes 0.25%	Trip Generation and Distribution				Total Site Trips	Full Build Volumes
				Enter Dist. %	Exit Dist. %	Trips IN 64	Trips OUT 46		
1	Gateway Blvd @ Northerly Driveway								
	SR								
	ST								
	SL								
	WR								
	WT								
	WL	29	29		10%			5	34
2	NR	24	24	10%		6		6	30
	NT								
	NL								
	ER								
	ET								
	EL								
	Gateway Blvd @ Proposed Driveway								
3	SR								
	ST								
	SL			15%				7	7
	WR			15%		10		10	10
	WT	24	24	10%		6		6	30
	WL								
	NR								
4	NT								
	NL								
	ER								
	ET	29	29		10%			5	34
	EL								
	Gateway Blvd @ Exiting Driveway								
	SR								
5	ST								
	SL	66	67		75%			34	101
	WR								
	WT	24	24	25%		16		16	40
	WL								
	NR								
	NT								
6	NL								
	ER								
	ET	29	29		25%			12	41
	EL								
	Gateway Blvd @ Entering Driveway/Southtowns Dwy								
	SR								
	ST								
7	SL								
	WR	22	22	75%		48		48	70
	WT	24	24	25%		16		16	40
	WL	3	3						3
	NR	31	31						31
	NT								
	NL								
8	ER								
	ET	94	95		100%			46	141
	EL	1	1						1

PROJECT DETAILS						
Project Name:	Gateway Building	Type of Project:		City:		
Project No:		Built-up Area(Sq.Ft):		Clients Name:		
Country:		ZIP/Postal Code:		No. of Scenarios:	2	
Analyst Name:	Amy Dake	Date:	5/22/2024	State/Province:		
Scenarios	Name	No. of Land Uses	Phases of Development	No. of Years to Project	User Group	
Scenario - 1	AM Peak Hour	3	1	0	Entry	Estimated New Vehicle Trips
Scenario - 2	PM Peak Hour	3	1	0	64	Exit
					22	Total
					55	77
					46	110

Scenario Name: AM Peak Hour
 Dev. phase: 1
 Analyst Note:

Warning:

VEHICLE TRIPS BEFORE REDUCTION

Land Use & Data Source	Location	IV	Size	Time Period	Method	Entry	Split%	Exit	Total
310 - Hotel Data Source: Trip Generation Manual, 11th Ed	General Urban/Suburban	Rooms	35	Weekday, Peak Hour of Adjacent Street Traffic, $T = 0.50(X) - 7.45$	Best Fit (L/N)	6	4	10	
221(1) - Multifamily Housing (Mid-Rise) - Not Data Source: Trip Generation Manual, 11th Ed	General Urban/Suburban	Dwelling Units	40	Weekday, Peak Hour of Adjacent Street Traffic, $T = 0.44(X) - 11.61$	Best Fit (L/N)	1	5	6	
215 - Single-Family Attached Housing Data Source: Trip Generation Manual, 11th Ed	General Urban/Suburban	Dwelling Units	128	Weekday, Peak Hour of Adjacent Street Traffic, $T = 0.52(X) - 5.70$	Best Fit (L/N)	15	46	61	
						25%	75%		

VEHICLE TO PERSON TRIP CONVERSION

BASELINE SITE VEHICLE CHARACTERISTICS:

Land Use	Baseline Site Vehicle Mode Share		Baseline Site Vehicle Occupancy Entry (%)	Baseline Site Vehicle Occupancy Exit (%)	Directional Split Entry (%)	Directional Split Exit (%)
	Entry (%)	Exit (%)				
310 - Hotel	100	100	1	1	56	44
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	100	100	1	1	23	77
215 - Single-Family Attached Housing	100	100	1	1	25	75

ESTIMATED BASELINE SITE PERSON TRIPS:

Land Use	Person Trips by Vehicle		Person Trips by Other Modes	Total Baseline Site Person Trips	
	Entry	Exit			
310 - Hotel	6	4	0	6	4
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	1	5	0	0	10
215 - Single-Family Attached Housing	15	46	0	15	6
	61	0	0	61	5

VEHICLE TRIPS AFTER MULTI-MODAL ADJUSTMENT

MODE SHARE:

Land Use	Personal Passenger-Vehicle		Truck	Entry (%)	Exit (%)	Other Modes
	Entry (%)	Exit (%)				
310 - Hotel	100%	100%	0%	0%	0%	0%
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	100%	100%	0%	0%	0%	0%
215 - Single-Family Attached Housing	100%	100%	0%	0%	0%	0%

OCCUPANCY:

Land Use	Entry		Vehicle	Exit
	Entry	Vehicle		
310 - Hotel	1.00			1.00
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit				1.00
215 - Single-Family Attached Housing				1.00

ADJUSTED VEHICLE TRIPS:

Land Use	Entry			Exit		
	Person Trips	Vehicle Mode Share (%)	Vehicle Occupancy	Person Trips	Vehicle Mode Share (%)	Vehicle Occupancy
310 - Hotel	6	100%	1.00	6	100%	1.00
221(1) - Multifamily Housing (Mid-Rise) -	1	100%	1.00	1	100%	1.00
215 - Single-Family Attached Housing	15	100%	1.00	15	100%	1.00
				46		46

INTERNAL VEHICLE TRIP REDUCTION**LAND USE GROUP ASSIGNMENT:**

Land Use	Land Use Group
310 - Hotel	Hotel
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	Residential
215 - Single-Family Attached Housing	Residential

BALANCED PERSON TRIPS:

310 - Hotel	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Persons Exit	4	0	0	0	0	0	0	1
Persons Entry	PAF	UIPTC	Unconstrained Demand	<<== BALANCED <<==	Unconstrained Demand	UIPTC	PAF	Persons Exit
6	0	0	0	0	0	0	0	5
310 - Hotel	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Persons Exit	4	0	0	0	0	0	0	15
Persons Entry	PAF	UIPTC	Unconstrained Demand	<<== BALANCED <<==	Unconstrained Demand	UIPTC	PAF	Persons Exit
6	0	0	0	0	0	0	0	46
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Persons Exit	5	0	0	0	0	0	0	15
Persons Entry	PAF	UIPTC	Unconstrained Demand	<<== BALANCED <<==	Unconstrained Demand	UIPTC	PAF	Persons Exit
1	0	0	0	0	0	0	0	46
215 - Single-Family Attached Housing	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Internal Person Trips From								
Total Internal Person Trips								
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Internal Person Trips From								
Total Internal Person Trips								
215 - Single-Family Attached Housing	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Internal Person Trips From								
Total Internal Person Trips								

INTERNAL PERSON TRIPS:

310 - Hotel	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Internal Person Trips From								
Total Internal Person Trips								
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Internal Person Trips From								
Total Internal Person Trips								
215 - Single-Family Attached Housing	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC	PAF	Persons Entry
Internal Person Trips From								
Total Internal Person Trips								

INTERNAL VEHICLE TRIPS AND CAPTURE:

310 - Hotel

Total Internal Person Trips	0	0	0	0
Vehicle Mode Share	100%	100%	100%	0
Vehicle Occupancy	-	-	-	-
Total Vehicle Internal Trips	0	0	0	0
Total External\Vehicle Trips	6	4	10	10
Internal Vehicle Trip Capture	0%	0%	0%	0%

221(1) - Multifamily Housing (Mid-Rise), Not Close to Rail Transit

Total Internal Person Trips	0	0	0	0
Vehicle Mode Share	100%	100%	100%	0
Vehicle Occupancy	-	-	-	-
Total Vehicle Internal Trips	0	0	0	0
Total External\Vehicle Trips	1	5	6	6
Internal Vehicle Trip Capture	0%	0%	0%	0%

215 - Single-Family Attached Housing

Total Internal Person Trips	0	0	0	0
Vehicle Mode Share	100%	100%	100%	0
Vehicle Occupancy	-	-	-	-
Total Vehicle Internal Trips	0	0	0	0
Total External\Vehicle Trips	15	46	61	61
Internal Vehicle Trip Capture	0%	0%	0%	0%

PASS-BY VEHICLE TRIP REDUCTION

Land Use	External Vehicle Trips		Pass-by Vehicle Trip %		Pass-by Vehicle Trips	
	Entry	Exit	Entry (%)	Exit (%)	Entry	Exit
310 - Hotel	6	4	0.00%	0.00%	0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	1	5	0.00%	0.00%	0	0
215 - Single-Family Attached Housing	15	46	0.00%	0.00%	0	0

DIVERTED VEHICLE TRIP REDUCTION

Land Use	External Vehicle Trips		Diverted Vehicle Trip %		Diverted Vehicle Trips	
	Entry	Exit	Entry (%)	Exit (%)	Entry	Exit
310 - Hotel	6	4	0.00%	0.00%	0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	1	5	0.00%	0.00%	0	0
215 - Single-Family Attached Housing	15	46	0.00%	0.00%	0	0

EXTRA VEHICLE TRIP REDUCTION

Land Use	(External - (Pass-by + Diverted)) Vehicle Trips		Extra Vehicle Trip Reduction %		Extra Reduced Vehicle Trips	
	Entry	Exit	Entry (%)	Exit (%)	Entry	Exit
310 - Hotel	6	4	0.00%	0.00%	0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	1	5	0.00%	0.00%	0	0
215 - Single-Family Attached Housing	15	46	0.00%	0.00%	0	0

NEW VEHICLE TRIPS

Land Use	New Vehicle Trips			Total
	Entry	Exit	Total	
310 - Hotel	6	4	10	
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	1	5	6	
215 - Single-Family Attached Housing	15	46	61	
Land Use	New Vehicle Trips (PPV)			
310 - Hotel	6	4	10	
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	1	5	6	
215 - Single-Family Attached Housing	15	46	61	
Land Use	New Vehicle Trips (Truck)			
310 - Hotel	0	0	0	
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	0	0	0	
215 - Single-Family Attached Housing	0	0	0	
RESULTS	New Vehicle Trips			
Site Totals	Entry	Exit	Total	
Vehicle Trips Before Reduction	22	55	77	
Vehicle Trips After Multi-modal Adjustment	22	55	77	
Internal Vehicle Trips	0	0	0	
External Vehicle Trips	22	55	77	
Internal Vehicle Trip Capture	0%	0%	0%	
Pass-by Vehicle Trips	0	0	0	
Diverted Vehicle Trips	0	0	0	
Extra Reduced Vehicle Trips	0	0	0	
New Vehicle Trips	22	55	77	
PPV	22	55	77	
Truck	0	0	0	
Person Trips by Other Modes	0	0	0	

Scenario - 2	User Group:
Dev. phase: 1	No. of Years to Project 0
Analyst Note:	
Warning:	

VEHICLE TRIPS BEFORE REDUCTION

Land Use & Data Source	Location	IV	Size	Time Period	Method	Entry	Split%	Exit	Total
310 - Hotel	General	Rooms	35	Weekday, Peak Hour of Adjacent Street Traffic,	Average	11	10	49%	21
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban				0.59	51%			
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	General	Dwelling Units	40	Weekday, Peak Hour of Adjacent Street Traffic, $T = 0.3(X) + 0.34$	Best Fit (LN)	10	6	39%	16
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban				Best Fit (LN)	43	30		
215 - Single-Family Attached Housing	General	Dwelling Units	128	Weekday, Peak Hour of Adjacent Street Traffic, $T = 0.6(X) - 3.93$	Best Fit (LN)	59%	41%		73
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban								

VEHICLE TO PERSON TRIP CONVERSION

BASELINE SITE VEHICLE CHARACTERISTICS:

Land Use	Baseline Site Vehicle Mode Share		Baseline Site Vehicle Occupancy	Exit (%)	Baseline Site Vehicle Directional Split
	Entry (%)	Exit (%)			
310 - Hotel	100	100	1	1	51
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	100	100	1	1	61
215 - Single-Family Attached Housing	100	100	1	1	59

ESTIMATED BASELINE SITE PERSON TRIPS:

Land Use	Person Trips by Vehicle		Person Trips by Other Modes	Total Baseline Site Person Trips
	Entry	Exit		
310 - Hotel	11	10	0	0
		21	0	10
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	10	6	0	21
		16	0	6
215 - Single-Family Attached Housing	43	30	0	16
		73	0	30
				73

VEHICLE TRIPS AFTER MULTI-MODAL ADJUSTMENT

MODE SHARE:

Land Use	Personal Passenger-Vehicle		Truck	Entry (%)	Exit (%)	Other Modes
	Entry (%)	Exit (%)				
310 - Hotel	100%	100%	0%	0%	0%	0%
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	100%	100%	0%	0%	0%	0%
215 - Single-Family Attached Housing	100%	100%	0%	0%	0%	0%

OCCUPANCY:

Land Use	Entry	Vehicle	Exit
310 - Hotel	1.00		1.00
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit			
215 - Single-Family Attached Housing	1.00		1.00

ADJUSTED VEHICLE TRIPS:

Land Use	Entry			Exit		
	Person Trips	Vehicle Mode Share (%)	Vehicle Occupancy	Person Trips	Vehicle Mode Share (%)	Vehicle Occupancy
310 - Hotel	11	100%	1.00	11	100%	1.00
221(1) - Multifamily Housing (Mid-Rise) -	10	100%	1.00	10	100%	1.00
215 - Single-Family Attached Housing	43	100%	1.00	43	100%	1.00

INTERNAL VEHICLE TRIP REDUCTION**LAND USE GROUP ASSIGNMENT:**

Land Use	Land Use Group
310 - Hotel	Hotel
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	Residential
215 - Single-Family Attached Housing	Residential

BALANCED PERSON TRIPS:

221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit						
Persons Exit	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC
10	0	0	0	0	0	0
Persons Entry	PAF	UIPTC	Unconstrained Demand	<<== BALANCED <<==	Unconstrained Demand	UIPTC
11	0	0	0	0	0	0
310 - Hotel						
Persons Exit	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC
10	0	0	0	0	0	0
Persons Entry	PAF	UIPTC	Unconstrained Demand	<<== BALANCED <<==	Unconstrained Demand	UIPTC
11	0	0	0	0	0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit						
Persons Exit	PAF	UIPTC	Unconstrained Demand	==>> BALANCED ==>>	Unconstrained Demand	UIPTC
6	0	0	0	0	0	0
Persons Entry	PAF	UIPTC	Unconstrained Demand	<<== BALANCED <<==	Unconstrained Demand	UIPTC
10	0	0	0	0	0	0
215 - Single-Family Attached Housing						
Internal Person Trips From			Entry		Exit	Total
Total Internal Person Trips			0		0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit						
Internal Person Trips From			Entry		Exit	Total
Total Internal Person Trips			0		0	0
215 - Single-Family Attached Housing						
Internal Person Trips From			Entry		Exit	Total
Total Internal Person Trips			0		0	0

INTERNAL PERSON TRIPS:

310 - Hotel						
Internal Person Trips From			Entry		Exit	Total
Total Internal Person Trips			0		0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit						
Internal Person Trips From			Entry		Exit	Total
Total Internal Person Trips			0		0	0
215 - Single-Family Attached Housing						
Internal Person Trips From			Entry		Exit	Total
Total Internal Person Trips			0		0	0

INTERNAL VEHICLE TRIPS AND CAPTURE:

310 - Hotel

Total Internal Person Trips	0	0	0	0
Vehicle Mode Share	100%	100%	100%	0
Vehicle Occupancy	-	-	-	-
Total Vehicle Internal Trips	0	0	0	0
Total External\Vehicle Trips	11	10	10	21
Internal Vehicle Trip Capture	0%	0%	0%	0%

221(1) - Multifamily Housing (Mid-Rise), Not Close to Rail Transit

Total Internal Person Trips	0	0	0	0
Vehicle Mode Share	100%	100%	100%	0
Vehicle Occupancy	-	-	-	-
Total Vehicle Internal Trips	0	0	0	0
Total External\Vehicle Trips	10	6	6	16
Internal Vehicle Trip Capture	0%	0%	0%	0%

215 - Single-Family Attached Housing

Total Internal Person Trips	0	0	0	0
Vehicle Mode Share	100%	100%	100%	0
Vehicle Occupancy	-	-	-	-
Total Vehicle Internal Trips	0	0	0	0
Total External\Vehicle Trips	43	30	30	73
Internal Vehicle Trip Capture	0%	0%	0%	0%

PASS-BY VEHICLE TRIP REDUCTION

Land Use	External Vehicle Trips		Pass-by Vehicle Trip %		Pass-by Vehicle Trips	Entry	Exit
	Entry	Exit	Entry (%)	Exit (%)			
310 - Hotel	11	10	0.00%	0.00%	0	0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	10	6	0.00%	0.00%	0	0	0
215 - Single-Family Attached Housing	43	30	0.00%	0.00%	0	0	0

DIVERTED VEHICLE TRIP REDUCTION

Land Use	External Vehicle Trips		Diverted Vehicle Trip %		Diverted Vehicle Trips	Entry	Exit
	Entry	Exit	Entry (%)	Exit (%)			
310 - Hotel	11	10	0.00%	0.00%	0	0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	10	6	0.00%	0.00%	0	0	0
215 - Single-Family Attached Housing	43	30	0.00%	0.00%	0	0	0

EXTRA VEHICLE TRIP REDUCTION

Land Use	(External - (Pass-by + Diverted)) Vehicle Trips		Extra Vehicle Trip Reduction %		Extra Reduced Vehicle Trips	Entry	Exit
	Entry	Exit	Entry (%)	Exit (%)			
310 - Hotel	11	10	0.00%	0.00%	0	0	0
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	10	6	0.00%	0.00%	0	0	0
215 - Single-Family Attached Housing	43	30	0.00%	0.00%	0	0	0

NEW VEHICLE TRIPS

Land Use	New Vehicle Trips			Total
	Entry	Exit	Total	
310 - Hotel	11	10	21	
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	10	6	16	
215 - Single-Family Attached Housing	43	30	73	
Land Use	New Vehicle Trips (PPV)			Total
	Entry	Exit	Total	
310 - Hotel	11	10	21	
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	10	6	16	
215 - Single-Family Attached Housing	43	30	73	
Land Use	New Vehicle Trips (Truck)			Total
	Entry	Exit	Total	
310 - Hotel	0	0	0	
221(1) - Multifamily Housing (Mid-Rise) - Not Close to Rail Transit	0	0	0	
215 - Single-Family Attached Housing	0	0	0	
RESULTS				
Site Totals	New Vehicle Trips			Total
	Entry	Exit	Total	
Vehicle Trips Before Reduction	64	46	110	
Vehicle Trips After Multi-modal Adjustment	64	46	110	
Internal Vehicle Trips	0	0	0	
External Vehicle Trips	64	46	110	
Internal Vehicle Trip Capture	0%	0%	0%	
Pass-by Vehicle Trips	0	0	0	
Diverted Vehicle Trips	0	0	0	
Extra Reduced Vehicle Trips	0	0	0	
New Vehicle Trips	64	46	110	
PPV	64	46	110	
Truck	0	0	0	
Person Trips by Other Modes	0	0	0	

PROJECT DETAILS							
Project Name:	Gateway Building Full Office	Type of Project:					
Project No:		City:					
Country:		Built-up Area(Sq.Ft):					
Analyst Name:	Amy Dake	Clients Name:					
Date:	6/6/2024	ZIP/Postal Code:					
State/Province:		No. of Scenarios:	2				
Analysis Region:							
SCENARIO SUMMARY							
Scenarios	Name	No. of Land Uses	Phases of Development	No. of Years to Project	User Group	Estimated Traffic	New Vehicle Trips
Scenario - 1	AM Peak	1	1	0		193	26
Scenario - 2	PM Peak	1	1	0		37	179
							Total
							219
							216

Scenario - 1	Scenario Name: AM Peak
Dev. phase: 1	No. of Years to Project 0
Analyst Note:	

Warning:

VEHICLE TRIPS BEFORE REDUCTION

Land Use & Data Source	Location	IV	Size	Time Period	Method	Entry	Exit	Total
710 - General Office Building	General	1000 Sq. Ft. GFA	137	Weekday, Peak Hour of Adjacent Street Traffic,	Rate/Equation	193	26	
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban			Ln(T)=0.36Ln(X)+1.16	Best Fit (LOG)	88%	12%	
								219

VEHICLE TO PERSON TRIP CONVERSION**BASELINE SITE VEHICLE CHARACTERISTICS:**

Land Use	Baseline Site Vehicle Mode Share	Baseline Site Vehicle Occupancy	Baseline Site Vehicle Directional Split
710 - General Office Building	99	100	
	99	100	
	99	100	

ESTIMATED BASELINE SITE PERSON TRIPS:

Land Use	Person Trips by Vehicle	Person Trips by Other Modes	Total Baseline Site Person Trips
710 - General Office Building	212	29	241
	212	29	241
	212	29	241

NEW VEHICLE TRIPS

Land Use	Entry	Exit	New Vehicle Trips	Total
710 - General Office Building	193	26	26	219
	193	26	26	219
	193	26	26	219

RESULTS

Site Totals	Entry	Exit	Total
Vehicle Trips Before Reduction	193	26	219
External Vehicle Trips	193	26	219
New Vehicle Trips	193	26	219

Scenario - 2	Scenario Name: PM Peak
Dev. phase: 1	No. of Years to Project 0
Analyst Note:	

Warning:

VEHICLE TRIPS BEFORE REDUCTION

Land Use & Data Source	Location	IV	Size	Time Period	Method	Entry	Exit	Total
Land Use	Entry (%)	Exit (%)	Mode Share	Baseline Site Vehicle Occupancy	Rate/Equation	Split%	Split%	
710 - General Office Building	General	1000 Sq. Ft. GFA	137	Weekday, Peak Hour of Adjacent Street Traffic,	Best Fit (LOG) $\ln(T) = 0.33\ln(X) + 1.29$	37	179	216
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban					17%	83%	

VEHICLE TO PERSON TRIP CONVERSION**BASELINE SITE VEHICLE CHARACTERISTICS:**

Land Use	Entry (%)	Exit (%)	Mode Share	Baseline Site Vehicle Occupancy	Exit	Entry (%)	Baseline Site Vehicle Directional Split	Exit (%)
710 - General Office Building	100	99		1.1	1.1	17	83	

ESTIMATED BASELINE SITE PERSON TRIPS:

Land Use	Entry	Exit	Person Trips by Vehicle	Person Trips by Other Modes	Total Baseline Site Person Trips	
Land Use	Entry	Exit	Entry	Exit	Entry	Exit
710 - General Office Building	40	197	237	2	40	199
					239	

NEW VEHICLE TRIPS

Land Use	Entry	Exit	New Vehicle Trips	Total
710 - General Office Building	37	179	179	216

RESULTS

Site Totals	Entry	Exit	Total
Vehicle Trips Before Reduction	37	179	216
External Vehicle Trips	37	179	216
New Vehicle Trips	37	179	216

Project: **Description:** **Gateway Building**
Residential Hotel Office

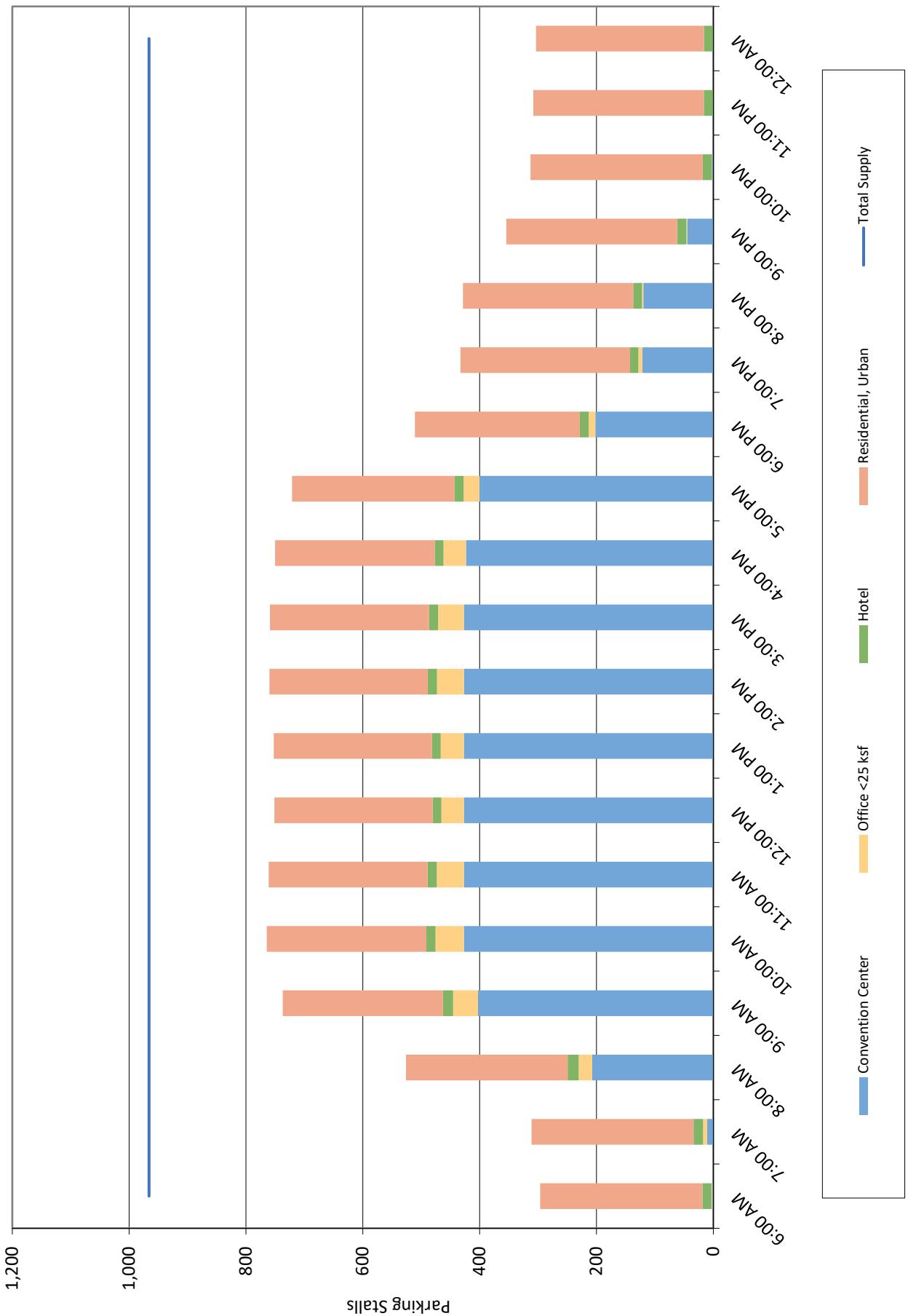
Shared Parking Reduction

Project: Gateway Building
Description: Residential, Hotel, Office

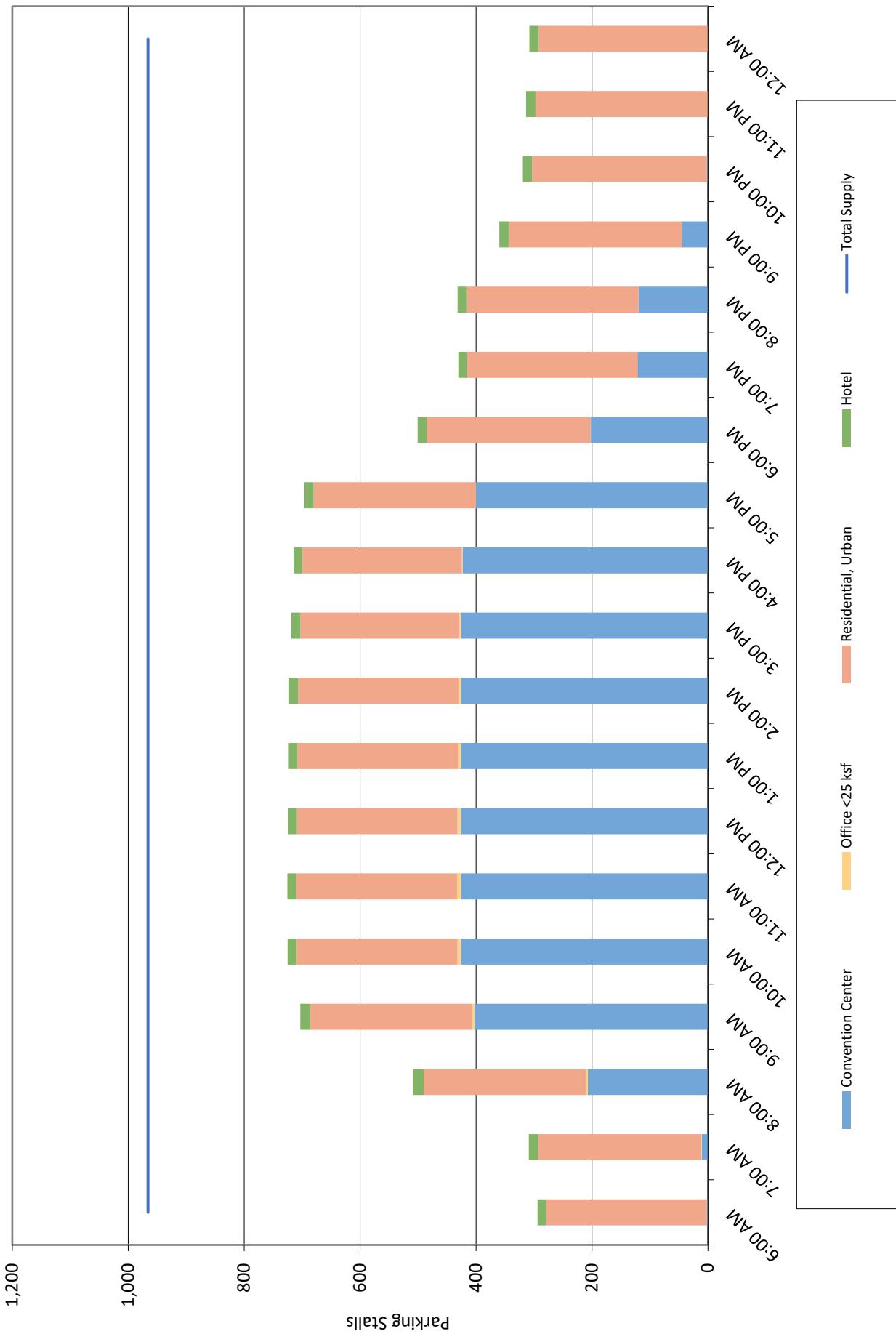
Month	Monthly Comparison Summary							
	Weekday							
	Overall Pk		AM Peak Hr		PM Peak Hr		Eve Peak Hr	
	Time	Demand	Time	Demand	Time	Demand	Time	Demand
January	10 AM	660	10 AM	660	2 PM	655	6 PM	460
February	10 AM	764	10 AM	764	2 PM	760	6 PM	511
March	10 AM	727	10 AM	727	2 PM	722	6 PM	494
April	10 AM	578	10 AM	578	2 PM	573	6 PM	423
May	10 AM	597	10 AM	597	2 PM	593	6 PM	431
June	10 AM	555	10 AM	555	2 PM	550	6 PM	411
July	10 AM	532	10 AM	532	2 PM	527	6 PM	401
August	10 AM	660	10 AM	660	2 PM	660	6 PM	462
September	10 AM	680	10 AM	680	2 PM	675	6 PM	469
October	10 AM	701	10 AM	701	2 PM	697	6 PM	479
November	10 AM	762	10 AM	762	2 PM	757	6 PM	508
December	10 AM	757	10 AM	757	2 PM	753	6 PM	504
Late December	10 AM	330	10 AM	330	2 PM	325	10 PM	313

Month	Monthly Comparison Summary							
	Weekend							
	Overall Pk		AM Peak Hr		PM Peak Hr		Eve Peak Hr	
	Time	Demand	Time	Demand	Time	Demand	Time	Demand
January	11 AM	621	11 AM	621	12 PM	619	6 PM	450
February	11 AM	725	11 AM	725	12 PM	724	6 PM	501
March	11 AM	688	11 AM	688	12 PM	686	6 PM	484
April	11 AM	539	11 AM	539	12 PM	537	6 PM	413
May	11 AM	558	11 AM	558	12 PM	557	6 PM	421
June	11 AM	516	11 AM	516	12 PM	514	6 PM	401
July	11 AM	495	11 AM	495	12 PM	493	6 PM	391
August	11 AM	623	11 AM	623	12 PM	621	6 PM	452
September	11 AM	641	11 AM	641	12 PM	640	6 PM	459
October	11 AM	662	11 AM	662	12 PM	661	6 PM	469
November	11 AM	723	11 AM	723	12 PM	721	6 PM	498
December	11 AM	718	11 AM	718	12 PM	717	6 PM	494
Late December	10 PM	319	8 AM	303	12 PM	298	10 PM	319

Peak Month Daily Parking Demand by Hour (Weekday)



Peak Month Daily Parking Demand by Hour (Weekend)



Project: Gateway Building
Description: Residential, Hotel, Office

Shared Parking Demand Summary																	
Land Use	Project Data			Weekday			Weekend			Weekday			Weekend				
	Quantity		Unit	Base Ratio	Driving Adj	Project Ratio	Unit For Ratio	Base Ratio	Driving Adj	Non-Captive Ratio	Project Ratio	Unit For Ratio	Peak Hr Adj	Peak Mo Adj	Estimated Parking Demand		
													10 AM	March			
Food and Beverage																	
Entertainment and Institutions																	
Hotel and Residential																	
Hotel-Business	35		keys	1.00	59%	100%	0.59	key	1.00	69%	100%	0.69	key	60%	90%		
Hotel-Leisure	35		keys	1.00	50%	100%	0.50	key	1.00	50%	100%	0.50	key	70%	100%		
Hotel Employees	35		keys	0.15	100%	100%	0.15	key	0.15	100%	100%	0.15	key	100%	100%		
Restaurant/Lounge	sf GLA		sf GLA	6.67	63%	90%	3.78	sf GLA	7.67	54%	30%	1.24	ksf GLA	10%	95%		
Meeting/Banquet (0 to 20 sq ft/key)	sf GLA		sf GLA	0.00	68%	60%	0.00	sf GLA	0.00	68%	70%	0.00	ksf GLA	60%	100%		
Meeting/Banquet (20 to 50 sq ft/key)	sf GLA		sf GLA	0.00	68%	60%	0.00	sf GLA	0.00	68%	70%	0.00	ksf GLA	60%	100%		
Meeting/Banquet (50 to 100 sq ft/key)	sf GLA		sf GLA	0.00	68%	60%	0.00	sf GLA	0.00	68%	70%	0.00	ksf GLA	60%	100%		
Meeting (100 to 200 sq ft/key)	sf GLA		sf GLA	0.00	68%	60%	0.00	sf GLA	5.50	68%	70%	2.62	ksf GLA	100%	90%		
Convention (> 200 sq ft/key)	sf GLA		sf GLA	5.50	68%	60%	2.24	sf GLA	5.50	68%	70%	2.62	ksf GLA	100%	90%		
Restaurant/Meeting Employees	sf GLA		sf GLA	0.00	100%	100%	0.00	sf GLA	0.00	100%	100%	0.00	ksf GLA	100%	100%		
Residential, Urban																	
Studio Efficiency																	
1 Bedroom	units		units	0.07	100%	100%	0.07	unit	0.07	100%	100%	0.07	unit	60%	100%		
2 Bedrooms	168		units	0.13	100%	100%	0.13	unit	0.13	100%	100%	0.13	unit	60%	100%		
3+ Bedrooms																	
Reserved	92%		res spaces	1.52	100%	100%	1.52	unit	1.52	100%	100%	1.52	unit	60%	100%		
Visitor	168		units	0.10	100%	100%	0.10	unit	0.15	100%	100%	0.15	unit	20%	100%		
Office <25 ksf	13,000		sf GFA	0.30	100%	100%	0.30	ksf GFA	0.03	100%	100%	0.03	ksf GFA	100%	100%		
Reserved Employee	empl		empl	0.00	100%	100%	0.00	empl	0.00	100%	100%	0.00	empl	100%	100%		
Additional Land Uses																	
Customer/Visitor	20																
Employee/Resident	64																
Reserved	256																
Total	339																
Shared Parking Reduction	12%																
	9%																

Shared Parking Reduction 12%

9%

Residential 17

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Project: Gateway Building
Description: Residential, Hotel, Office

Month	Monthly Comparison Summary							
	Weekday							
	Overall Pk		AM Peak Hr		PM Peak Hr		Eve Peak Hr	
	Time	Demand	Time	Demand	Time	Demand	Time	Demand
January	10 AM	336	10 AM	336	2 PM	331	7 PM	310
February	10 AM	338	10 AM	338	2 PM	333	7 PM	311
March	10 AM	339	10 AM	339	2 PM	335	10 PM	313
April	10 AM	339	10 AM	339	2 PM	335	10 PM	313
May	10 AM	338	10 AM	338	2 PM	333	7 PM	311
June	10 AM	338	10 AM	338	2 PM	333	7 PM	311
July	10 AM	336	10 AM	336	2 PM	332	10 PM	311
August	10 AM	336	10 AM	336	2 PM	336	10 PM	311
September	10 AM	335	10 AM	335	2 PM	330	7 PM	309
October	10 AM	335	10 AM	335	2 PM	330	7 PM	309
November	10 AM	335	10 AM	335	2 PM	330	7 PM	309
December	10 AM	331	10 AM	331	2 PM	326	7 PM	305
Late December	10 AM	330	10 AM	330	2 PM	325	10 PM	313

Month	Monthly Comparison Summary							
	Weekend							
	Overall Pk		AM Peak Hr		PM Peak Hr		Eve Peak Hr	
	Time	Demand	Time	Demand	Time	Demand	Time	Demand
January	10 PM	316	8 AM	300	12 PM	295	10 PM	316
February	10 PM	317	8 AM	302	12 PM	297	10 PM	317
March	10 PM	319	8 AM	304	12 PM	299	10 PM	319
April	10 PM	319	8 AM	304	12 PM	299	10 PM	319
May	10 PM	317	8 AM	302	12 PM	297	10 PM	317
June	10 PM	317	8 AM	302	12 PM	297	10 PM	317
July	10 PM	317	8 AM	303	12 PM	297	10 PM	317
August	10 PM	317	8 AM	303	12 PM	297	10 PM	317
September	10 PM	315	8 AM	299	12 PM	295	10 PM	315
October	10 PM	315	8 AM	299	12 PM	295	10 PM	315
November	10 PM	315	8 AM	299	12 PM	295	10 PM	315
December	10 PM	310	8 AM	294	12 PM	290	10 PM	310
Late December	10 PM	319	8 AM	303	12 PM	298	10 PM	319

Peak Month Daily Parking Demand by Hour (Weekday)



Peak Month Daily Parking Demand by Hour (Weekend)



APPENDIX C: LOS CALCULATIONS – EXISTING CONDITIONS

Intersection

Int Delay, s/veh 3.8

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	8	29	0	27	1
Future Vol, veh/h	0	8	29	0	27	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	2	0	0	2	8	0
Mvmt Flow	0	10	37	0	35	1

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	-
Stage 1	-	-	37
Stage 2	-	-	10
Critical Hdwy	-	-	6.48
Critical Hdwy Stg 1	-	-	5.48
Critical Hdwy Stg 2	-	-	5.48
Follow-up Hdwy	-	-	3.572
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	970
Stage 2	0	-	998
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	948
Mov Cap-2 Maneuver	-	-	948
Stage 1	-	-	970
Stage 2	-	-	998

Approach EB WB SB

HCM Control Delay, s/v 0 0 8.9

HCM LOS A

Minor Lane/Major Mvmt	EBT	WBT	SBLn1
Capacity (veh/h)	-	-	951
HCM Lane V/C Ratio	-	-	0.038
HCM Control Delay (s/veh)	-	-	8.9
HCM Lane LOS	-	-	A
HCM 95th %tile Q (veh)	-	-	0.1

Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	34	1	37	28	46	1	0	7	0	0	0
Future Vol, veh/h	0	34	1	37	28	46	1	0	7	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	6	0	24	0	4	0	0	0	2	2	2
Mvmt Flow	0	44	1	47	36	59	1	0	9	0	0	0

Major/Minor	Major1	Major2		Minor1				
Conflicting Flow All	95	0	0	45	0	0	205	234
Stage 1	-	-	-	-	-	-	45	45
Stage 2	-	-	-	-	-	-	160	189
Critical Hdwy	4.12	-	-	4.34	-	-	6.4	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.5
Follow-up Hdwy	2.218	-	-	2.416	-	-	3.5	4
Pot Cap-1 Maneuver	1499	-	-	1433	-	-	788	670
Stage 1	-	-	-	-	-	-	983	861
Stage 2	-	-	-	-	-	-	874	748
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1499	-	-	1433	-	-	760	0
Mov Cap-2 Maneuver	-	-	-	-	-	-	760	0
Stage 1	-	-	-	-	-	-	983	0
Stage 2	-	-	-	-	-	-	843	0

Approach	EB	WB		NB			
HCM Control Delay, s/v	0	2.5		8.7			
HCM LOS				A			
<hr/>							
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR
Capacity (veh/h)	987	1499	-	-	1433	-	-
HCM Lane V/C Ratio	0.01	-	-	-	0.033	-	-
HCM Control Delay (s/veh)	8.7	0	-	-	7.6	0	-
HCM Lane LOS	A	A	-	-	A	A	-
HCM 95th %tile Q (veh)	0	0	-	-	0.1	-	-

Intersection

Int Delay, s/veh 5.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	29	24	0	66	0
Future Vol, veh/h	0	29	24	0	66	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	73	73	73	73	73	73
Heavy Vehicles, %	2	7	4	2	2	0
Mvmt Flow	0	40	33	0	90	0

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	-
Stage 1	-	-	-
Stage 2	-	-	40
Critical Hdwy	-	-	6.42
Critical Hdwy Stg 1	-	-	5.42
Critical Hdwy Stg 2	-	-	5.42
Follow-up Hdwy	-	-	3.518
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	982
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	931
Mov Cap-2 Maneuver	-	-	931
Stage 1	-	-	989
Stage 2	-	-	982

Approach	EB	WB	SB
HCM Control Delay, s/v	0	0	9.3
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	SBLn1
Capacity (veh/h)	-	-	931
HCM Lane V/C Ratio	-	-	0.097
HCM Control Delay (s/veh)	-	-	9.3
HCM Lane LOS	-	-	A
HCM 95th %tile Q (veh)	-	-	0.3

Intersection

Int Delay, s/veh 1.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	1	94	0	3	24	22	0	0	31	0	0	0
Future Vol, veh/h	1	94	0	3	24	22	0	0	31	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	0	3	0	33	4	0	0	0	3	2	2	2
Mvmt Flow	1	129	0	4	33	30	0	0	42	0	0	0

Major/Minor	Major1	Major2		Minor1				
Conflicting Flow All	63	0	0	129	0	0	187	202
Stage 1	-	-	-	-	-	-	131	131
Stage 2	-	-	-	-	-	-	56	71
Critical Hdwy	4.1	-	-	4.43	-	-	6.4	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.5
Follow-up Hdwy	2.2	-	-	2.497	-	-	3.5	4
Pot Cap-1 Maneuver	1553	-	-	1286	-	-	807	698
Stage 1	-	-	-	-	-	-	900	792
Stage 2	-	-	-	-	-	-	972	840
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1553	-	-	1286	-	-	804	0
Mov Cap-2 Maneuver	-	-	-	-	-	-	804	0
Stage 1	-	-	-	-	-	-	899	0
Stage 2	-	-	-	-	-	-	969	0

Approach	EB	WB		NB				
HCM Control Delay, s/v	0.1	0.5		9.1				
HCM LOS				A				
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	
Capacity (veh/h)	918	1553	-	-	1286	-	-	
HCM Lane V/C Ratio	0.046	0.001	-	-	0.003	-	-	
HCM Control Delay (s/veh)	9.1	7.3	0	-	7.8	0	-	
HCM Lane LOS	A	A	A	-	A	A	-	
HCM 95th %tile Q (veh)	0.1	0	-	-	0	-	-	

APPENDIX D: LOS CALCULATIONS – BACKGROUND CONDITIONS

Intersection

Int Delay, s/veh 3.8

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	8	29	0	27	1
Future Vol, veh/h	0	8	29	0	27	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	2	0	0	2	8	0
Mvmt Flow	0	10	37	0	35	1

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	-
Stage 1	-	-	37
Stage 2	-	-	10
Critical Hdwy	-	-	6.48
Critical Hdwy Stg 1	-	-	5.48
Critical Hdwy Stg 2	-	-	5.48
Follow-up Hdwy	-	-	3.572
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	970
Stage 2	0	-	998
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	948
Mov Cap-2 Maneuver	-	-	948
Stage 1	-	-	970
Stage 2	-	-	998

Approach	EB	WB	SB
HCM Control Delay, s/v	0	0	8.9
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	SBLn1
Capacity (veh/h)	-	-	951
HCM Lane V/C Ratio	-	-	0.038
HCM Control Delay (s/veh)	-	-	8.9
HCM Lane LOS	-	-	A
HCM 95th %tile Q (veh)	-	-	0.1

Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	34	1	37	28	47	1	0	7	0	0	0
Future Vol, veh/h	0	34	1	37	28	47	1	0	7	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	6	0	24	0	4	0	0	0	2	2	2
Mvmt Flow	0	44	1	47	36	60	1	0	9	0	0	0

Major/Minor	Major1	Major2		Minor1				
Conflicting Flow All	96	0	0	45	0	0	205	235
Stage 1	-	-	-	-	-	-	45	45
Stage 2	-	-	-	-	-	-	160	190
Critical Hdwy	4.12	-	-	4.34	-	-	6.4	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.5
Follow-up Hdwy	2.218	-	-	2.416	-	-	3.5	4
Pot Cap-1 Maneuver	1498	-	-	1433	-	-	788	669
Stage 1	-	-	-	-	-	-	983	861
Stage 2	-	-	-	-	-	-	874	747
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1498	-	-	1433	-	-	760	0
Mov Cap-2 Maneuver	-	-	-	-	-	-	760	0
Stage 1	-	-	-	-	-	-	983	0
Stage 2	-	-	-	-	-	-	843	0

Approach	EB	WB		NB			
HCM Control Delay, s/v	0	2.5		8.7			
HCM LOS				A			
<hr/>							
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR
Capacity (veh/h)	987	1498	-	-	1433	-	-
HCM Lane V/C Ratio	0.01	-	-	-	0.033	-	-
HCM Control Delay (s/veh)	8.7	0	-	-	7.6	0	-
HCM Lane LOS	A	A	-	-	A	A	-
HCM 95th %tile Q (veh)	0	0	-	-	0.1	-	-

Intersection

Int Delay, s/veh 5.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	29	24	0	67	0
Future Vol, veh/h	0	29	24	0	67	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	73	73	73	73	73	73
Heavy Vehicles, %	2	7	4	2	2	0
Mvmt Flow	0	40	33	0	92	0

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	-
Stage 1	-	-	-
Stage 2	-	-	40
Critical Hdwy	-	-	6.42
Critical Hdwy Stg 1	-	-	5.42
Critical Hdwy Stg 2	-	-	5.42
Follow-up Hdwy	-	-	3.518
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	982
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	931
Mov Cap-2 Maneuver	-	-	931
Stage 1	-	-	989
Stage 2	-	-	982

Approach	EB	WB	SB
HCM Control Delay, s/v	0	0	9.3
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	SBLn1
Capacity (veh/h)	-	-	931
HCM Lane V/C Ratio	-	-	0.099
HCM Control Delay (s/veh)	-	-	9.3
HCM Lane LOS	-	-	A
HCM 95th %tile Q (veh)	-	-	0.3

Intersection

Int Delay, s/veh 1.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	1	95	0	3	24	22	0	0	31	0	0	0
Future Vol, veh/h	1	95	0	3	24	22	0	0	31	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	0	3	0	33	4	0	0	0	3	2	2	2
Mvmt Flow	1	130	0	4	33	30	0	0	42	0	0	0

Major/Minor	Major1	Major2		Minor1				
Conflicting Flow All	63	0	0	130	0	0	188	203
Stage 1	-	-	-	-	-	-	132	132
Stage 2	-	-	-	-	-	-	56	71
Critical Hdwy	4.1	-	-	4.43	-	-	6.4	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.5
Follow-up Hdwy	2.2	-	-	2.497	-	-	3.5	4
Pot Cap-1 Maneuver	1553	-	-	1285	-	-	806	697
Stage 1	-	-	-	-	-	-	899	791
Stage 2	-	-	-	-	-	-	972	840
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1553	-	-	1285	-	-	803	0
Mov Cap-2 Maneuver	-	-	-	-	-	-	803	0
Stage 1	-	-	-	-	-	-	898	0
Stage 2	-	-	-	-	-	-	969	0

Approach	EB	WB		NB				
HCM Control Delay, s/v	0.1	0.5		9.1				
HCM LOS				A				
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	
Capacity (veh/h)	917	1553	-	-	1285	-	-	
HCM Lane V/C Ratio	0.046	0.001	-	-	0.003	-	-	
HCM Control Delay (s/veh)	9.1	7.3	0	-	7.8	0	-	
HCM Lane LOS	A	A	A	-	A	A	-	
HCM 95th %tile Q (veh)	0.1	0	-	-	0	-	-	

APPENDIX E: LOS CALCULATIONS – FULL BUILD CONDITION

Intersection

Int Delay, s/veh 1.3

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	14	31	3	8	0
Future Vol, veh/h	0	14	31	3	8	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	2	2	2
Mvmt Flow	0	15	34	3	9	0

Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	37	0	-	0	51	36
Stage 1	-	-	-	-	36	-
Stage 2	-	-	-	-	15	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1574	-	-	-	958	1037
Stage 1	-	-	-	-	986	-
Stage 2	-	-	-	-	1008	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1574	-	-	-	958	1037
Mov Cap-2 Maneuver	-	-	-	-	958	-
Stage 1	-	-	-	-	986	-
Stage 2	-	-	-	-	1008	-

Approach	EB	WB	SB
HCM Control Delay, s/v	0	0	8.8
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1574	-	-	-	958
HCM Lane V/C Ratio	-	-	-	-	0.009
HCM Control Delay (s/veh)	0	-	-	-	8.8
HCM Lane LOS	A	-	-	-	A
HCM 95th %tile Q (veh)	0	-	-	-	0

Intersection

Int Delay, s/veh 5.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	22	34	0	68	1
Future Vol, veh/h	0	22	34	0	68	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	2	0	0	2	8	0
Mvmt Flow	0	28	44	0	87	1

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	-	0	-	0	72 44
Stage 1	-	-	-	-	44 -
Stage 2	-	-	-	-	28 -
Critical Hdwy	-	-	-	-	6.48 6.2
Critical Hdwy Stg 1	-	-	-	-	5.48 -
Critical Hdwy Stg 2	-	-	-	-	5.48 -
Follow-up Hdwy	-	-	-	-	3.572 3.3
Pot Cap-1 Maneuver	0	-	-	0	917 1032
Stage 1	0	-	-	0	963 -
Stage 2	0	-	-	0	979 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	917 1032
Mov Cap-2 Maneuver	-	-	-	-	917 -
Stage 1	-	-	-	-	963 -
Stage 2	-	-	-	-	979 -

Approach	EB	WB	SB
HCM Control Delay, s/v	0	0	9.3
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	SBLn1
Capacity (veh/h)	-	-	918
HCM Lane V/C Ratio	-	-	0.096
HCM Control Delay (s/veh)	-	-	9.3
HCM Lane LOS	-	-	A
HCM 95th %tile Q (veh)	-	-	0.3

4: Southtowns Driveway/Entering Driveway & Gateway Blvd

Intersection

Int Delay, s/veh 1.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	89	1	37	33	64	1	0	7	0	0	0
Future Vol, veh/h	0	89	1	37	33	64	1	0	7	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	78	78	78	78	78	78	78	78	78	78	78	78
Heavy Vehicles, %	2	6	0	24	0	4	0	0	0	2	2	2
Mvmt Flow	0	114	1	47	42	82	1	0	9	0	0	0

Major/Minor	Major1	Major2		Minor1				
Conflicting Flow All	124	0	0	115	0	0	292	333
Stage 1	-	-	-	-	-	-	115	115
Stage 2	-	-	-	-	-	-	177	218
Critical Hdwy	4.12	-	-	4.34	-	-	6.4	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.5
Follow-up Hdwy	2.218	-	-	2.416	-	-	3.5	4
Pot Cap-1 Maneuver	1463	-	-	1348	-	-	703	590
Stage 1	-	-	-	-	-	-	915	804
Stage 2	-	-	-	-	-	-	859	726
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1463	-	-	1348	-	-	676	0
Mov Cap-2 Maneuver	-	-	-	-	-	-	676	0
Stage 1	-	-	-	-	-	-	915	0
Stage 2	-	-	-	-	-	-	826	0

Approach	EB	WB		NB				
HCM Control Delay, s/v	0	2.1		9.1				
HCM LOS				A				
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	
Capacity (veh/h)	899	1463	-	-	1348	-	-	
HCM Lane V/C Ratio	0.011	-	-	-	0.035	-	-	
HCM Control Delay (s/veh)	9.1	0	-	-	7.8	0	-	
HCM Lane LOS	A	A	-	-	A	A	-	
HCM 95th %tile Q (veh)	0	0	-	-	0.1	-	-	

Intersection

Int Delay, s/veh 0.8

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	34	30	10	7	0
Future Vol, veh/h	0	34	30	10	7	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	7	4	2	2	2
Mvmt Flow	0	37	33	11	8	0

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	44	0	-
Stage 1	-	-	39
Stage 2	-	-	37
Critical Hdwy	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	5.42
Critical Hdwy Stg 2	-	-	5.42
Follow-up Hdwy	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	1564	-	927 1033
Stage 1	-	-	983
Stage 2	-	-	985
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1564	-	927 1033
Mov Cap-2 Maneuver	-	-	927
Stage 1	-	-	983
Stage 2	-	-	985

Approach	EB	WB	SB
HCM Control Delay, s/v	0	0	8.9
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1564	-	-	-	927
HCM Lane V/C Ratio	-	-	-	-	0.008
HCM Control Delay (s/veh)	0	-	-	-	8.9
HCM Lane LOS	A	-	-	-	A
HCM 95th %tile Q (veh)	0	-	-	-	0

Intersection

Int Delay, s/veh 5.4

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	41	40	0	101	0
Future Vol, veh/h	0	41	40	0	101	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	73	73	73	73	73	73
Heavy Vehicles, %	2	7	4	2	2	0
Mvmt Flow	0	56	55	0	138	0

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	-	0	-	0	111 55
Stage 1	-	-	-	-	55 -
Stage 2	-	-	-	-	56 -
Critical Hdwy	-	-	-	-	6.42 6.2
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	-	-	3.518 3.3
Pot Cap-1 Maneuver	0	-	-	0	886 1018
Stage 1	0	-	-	0	968 -
Stage 2	0	-	-	0	967 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	886 1018
Mov Cap-2 Maneuver	-	-	-	-	886 -
Stage 1	-	-	-	-	968 -
Stage 2	-	-	-	-	967 -

Approach EB WB SB

HCM Control Delay, s/v 0 0 9.8

HCM LOS A

Minor Lane/Major Mvmt	EBT	WBT	SBLn1
Capacity (veh/h)	-	-	886
HCM Lane V/C Ratio	-	-	0.156
HCM Control Delay (s/veh)	-	-	9.8
HCM Lane LOS	-	-	A
HCM 95th %tile Q (veh)	-	-	0.6

Intersection

Int Delay, s/veh 1.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	1	141	0	3	40	70	0	0	31	0	0	0
Future Vol, veh/h	1	141	0	3	40	70	0	0	31	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	0	3	0	33	4	0	0	0	3	2	2	2
Mvmt Flow	1	193	0	4	55	96	0	0	42	0	0	0

Major/Minor	Major1	Major2		Minor1				
Conflicting Flow All	151	0	0	193	0	0	306	354
Stage 1	-	-	-	-	-	-	195	195
Stage 2	-	-	-	-	-	-	111	159
Critical Hdwy	4.1	-	-	4.43	-	-	6.4	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.5
Follow-up Hdwy	2.2	-	-	2.497	-	-	3.5	4
Pot Cap-1 Maneuver	1442	-	-	1215	-	-	690	574
Stage 1	-	-	-	-	-	-	843	743
Stage 2	-	-	-	-	-	-	919	770
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1442	-	-	1215	-	-	687	0
Mov Cap-2 Maneuver	-	-	-	-	-	-	687	0
Stage 1	-	-	-	-	-	-	842	0
Stage 2	-	-	-	-	-	-	915	0

Approach	EB	WB		NB			
HCM Control Delay, s/v	0.1	0.2		9.5			
HCM LOS				A			
<hr/>							
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR
Capacity (veh/h)	846	1442	-	-	1215	-	-
HCM Lane V/C Ratio	0.05	0.001	-	-	0.003	-	-
HCM Control Delay (s/veh)	9.5	7.5	0	-	8	0	-
HCM Lane LOS	A	A	A	-	A	A	-
HCM 95th %tile Q (veh)	0.2	0	-	-	0	-	-